

## **Structural Design Calculations**

## **Heavy Duty Washout Pans**

Prepared For: Consolidated Fabricators Corp.

Project No.: 18-668 Date: June 26, 2018

### **Table of Contents**

# Structural Design Calculations for Heavy Duty Washout Pans

Devco Job # 18-668 June 26, 2018

Subject	Page
Design Criteria	1
Discussion of RISA 3D Results	2
Padeye Design	3-4
Risa 3D Model Results	Appendix.





Physical Address 245 NE Conifer Blvd. Corvallis, OR 97330 Mailing Address P.O. BOX 1211 Corvallis, OR 97339

www.devcoengineering.com

(541) 757-8991 Fax: (541) 757-9885

PROJECT:

Con-fab Washout Pans

PROJECT NO: 18-668

DESIGN: JC

**DATE: 6/18** 

#### Design Criteria:

The design criteria of the washout pans includes the maximum allowable stresses provided by the AISC Steel Construction Manual Edition 14 and by the ASME Design of Below the Hook Lifting Devices 2014.

AISC Requirements

(Steel Construction Manual Fourteenth Edition)

AISC requires the following factors of safety:

1.67 on Flexural Yielding1.5 on Shear Connections

Ref. pg 16.1-46 Ref. pg 16.1-129

2.0 on Block Shear Connections

Ref. pg 16.1-129

2.0 on Welded Connections

Ref. pg 16.1-115

**ASME Requirements** 

(ASME BTH-1-2014)

ASME requires the following factors of safety:

2.0 on Flexural Yielding

Ref. pg 10

3.17 on Shear Connections

Ref. pg 16

Block shear is undefined

n/a

2.4 on Welded Connections

Ref. pg 17

#### **Design Loads:**

Design loads include the dead load of the pans and the hydrostatic live load of 150 lbs/cu. ft. at the full volume capacity of the pans.

84"x84"x24" Pan

DL= 1,300 lbs

LL= 587 gallon capacity = 78.5 cu. ft. x 150 lbs/cu. ft. = 11,775 lbs

84"x84"x14" Pan

DL= 930 lbs

LL= 271 gallon capacity = 36.2 cu. ft. x 150 lbs/cu. ft. = 5,430 lbs

#### Additional Information:

Only the design of the washout pans and lifting eyes has been considered. Design of elements and components of the rigging system is by others. The lifting configuration must load all (4) lifting eyes equally and at a minimum angle of 48 degrees measured from the horizontal.



**Physical Address** 245 NE Conifer Blvd. Corvallis, OR 97330

www.devcoengineering.com

Mailing Address P.O. BOX 1211 Corvallis, OR 97339

(541) 757-8991 Fax: (541) 757-9885

PROJECT:

Con-fab Washout Pans

PROJECT NO: 18-668

DESIGN: JC

**DATE: 6/18** 

#### Discussion of RISA 3D Results:

Finite element modeling (RISA 3D) was used to analyze the stresses in the washout pan created by a single pick point overhead lift. The maximum allowable stress (per the design criteria section of this calculation package) is equal to the yield strength of the material divided by a factor of safety of 2.0. In this case:

Fy/2.0 = (36 ksi)/2.0 = 18 ksi (Gr. A36 Steel)

Both the 84x84x24" pan and the 84x84x14" pan were limited in strength by flexural yielding in the base of the pan.

#### 84x84x24" Pan:

Per the RISA 3D analysis, the maximum stress created by the design loads is 17 ksi, located along the extreme bending fibers of the base stiffeners. This falls below the maximum allowable stress of 18 ksi, therefore is ok for use at the rated live load of 12,000 lbs.

#### 84x84x14" Pan:

Per the RISA 3D analysis, the average maximum stress created by the design loads is 14.8 ksi, located in the center of the base and along the perimeter of the base. Additionally, there are localized maximum stresses around the fork lift pockets up to 24.8 ksi. These localized maximum stresses are created by the additional stiffness of the fork lift pockets and since they fall within the elastic deformation region of the steel, they can be negated, as any deflection around the area will cause a redistribution of stresses.

By comparison of the average maximum stress of 14.8 ksi to the 18 ksi maximum allowable stress, this pan is ok for use at the rated live load of 5,500 lbs.



**Physical Address** 245 NE Conifer Blvd. Corvallis, OR 97330 Mailing Address P.O. BOX 1211 Corvallis, OR 97339

www.devcoenglneering.com

**(541) 757-8991** Fax: (541) 757-9885

PROJECT: Con-tab Washout Pans	PROJECT NO:  8-648	DESIGN: JC	DATE: 6/18
Padeye Check			
0	Prox		
Proce = DL max + Ll max	48°	115/8"	
= 1300 16s + 12,000 15s	6.7"	5" 0	
= 3.4 k	6.7"	11/8" \$	
		3/4" br. 50 A	
Shear Yielding			
Rn = 0.6 Fy Anv (ATSC 34-4)	1		
Fy = 50 ks: Anv = 2 (3/4")(15/11") = 1.97	) 1, 2		
Rn = 59 kips			
$R_a = \frac{R_n}{FS = 3.17} = 18.6 k > P_{ma}$	v <u>Voh</u> (FS= F7	)	
Bloch Shear Rupture			
Rn = O. 6 Fy Agy + Uss Fu Ant	(AISC JY-5)		
Fy = 50 hsi Agu = Fu = 60 hsi Ant = Vss =		Sin <sup>2</sup>	
Rn = 88.6 kips			
$R_{\alpha} = \frac{R_{n}}{F_{s}=2} = 44 k > P_{ma}$	× Vok (FS= ?	26)	
		PAGE 3	OF



**Physical Address** 245 NE Conifer Bivd. Corvallis, OR 97330 Mailing Address P.O. BOX 1211 Corvallis, OR 97339

www.devcoengineering.com

(541) 757-8991 Fax: (541) 757-9885

PROJECT:	<u>dege</u> <u>Weld</u>	Chec	Washoulk (Cont.		5	PROJECT N	o: 18·	668	DESIGN:	20	DATE:	6/18
<u>Pa</u>	Weld		k (Cont	2								
	Weld			2								
	25 =	<b>D</b> .										
		2(1	sin (48)	=	0.19 k	-/in						
	9× =	Pmax	(48) (4.7")	:	0.171	yin						
	9m×		× (3.5")	a	0.795	re/in						
	Q <sub>r</sub> =	24 +	(qx2+qmx	) <sup>/2</sup>	= 1.0	k/in						
	Try	Y4" F1	llet Weld	(Min	mum)	?						
	= =	0.6F	Exx × 0.7 (70 hsi) 0. - k/in	07 a		3)						
	Ra	= Rn FS= :	2.4	3.09	k/in 3	,	<u>Joh</u>	(FS	- 7.4)			
			Padey	و ٥	<u>K</u>							
			Limit	ns Ib	lure	= Weld	(FS:	= 7 4	)			
		<b> </b>		2 1 "	-1 (00 )	- 014	\ 1		_1			
									PAG	ie 4	OF	











