



Technical Bulletin

Updated 23-03-25

Skyline Modular Joist Reinforcer System (Patents Pending)

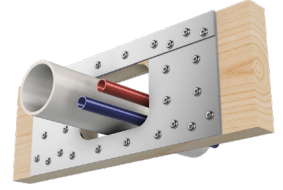
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Joist Reinforcer Guide (Patents Pending)

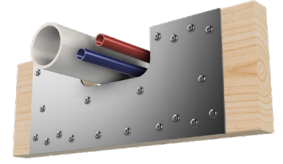
Hole Reinforcers

| Model No | Joist Size | Gauge | Dimensions (in) | | Maximum Hole Size (in) | | Fastener Qty | No of Components |
|----------|------------------|-------|-----------------|--------|------------------------|------|--------------|------------------|
| | | | H | W | H | W | | |
| 2x10 HR | 2" x 10" Nominal | 10 | 9.00" | 15.75" | 7.50" | 6.0" | 33 | 3 |
| 2x8 HR | 2" x 8" Nominal | 10 | 7.00" | 15.75" | 6.00" | 6.0" | 31 | 3 |
| 2x6 HR | 2" x 6" Nominal | 10 | 5.50" | 15.75" | 2.50" | 6.0" | 29 | 3 |



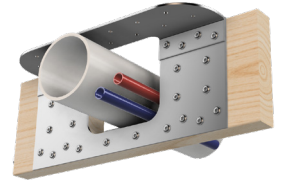
Shallow Notch Reinforcers

| Model No | Joist Size | Gauge | Dimensions (in) | | Maximum Notch Size (in) | | Fastener Qty | No of Components |
|----------|------------------|-------|-----------------|--------|-------------------------|------|--------------|------------------|
| | | | H | W | H | W | | |
| 2x10 SNR | 2" x 10" Nominal | 10 | 9.0" | 15.75" | 3.0" | 6.0" | 22 | 1 |
| 2x8 SNR | 2" x 8" Nominal | 10 | 7.0" | 15.75" | 3.0" | 6.0" | 26 | 1 |



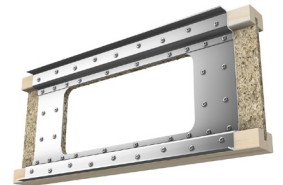
Deep Notch Reinforcers

| Model No | Joist Size | Gauge | Dimensions (in) | | Maximum Notch Size (in) | | Fastener Qty | No of Components |
|----------|------------------|-------|-----------------|--------|-------------------------|-------|--------------|------------------|
| | | | H | W | H | W | | |
| 2x10 DNR | 2" x 10" Nominal | 10 | 9.00" | 15.75" | 7.50" | 6.00" | 36 | 2 |
| 2x8 DNR | 2" x 8" Nominal | 10 | 7.00" | 15.75" | 5.50" | 6.00" | 36 | 2 |
| 2x6 DNR | 2" x 6" Nominal | 10 | 5.50" | 15.75" | 4.00" | 6.00" | 36 | 2 |



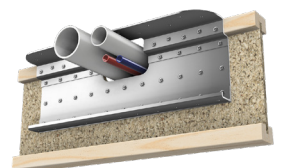
iJoist Web Reinforcers

| Model No | Approximate Flange Width (in) | Overall iJoist Height (in) | Gauge | Dimensions (in) | | Maximum Opening (in) | | Fastener Qty | No of Components |
|-----------|-------------------------------|----------------------------|-------|-----------------|--------|----------------------|-----|--------------|------------------|
| | | | | H | W | H | W | | |
| 150 WR 95 | 1.50" | 9.50" | 10 | 9.50" | 23.75" | 4.75" | 16" | 42 | 6 |
| 150 WR 11 | | 11.875" | 10 | 11.875" | 23.75" | 7.125" | 16" | 42 | 6 |
| 150 WR 14 | | 14.00" | 10 | 14.00" | 23.75" | 9.25" | 16" | 48 | 6 |
| 150 WR 16 | | 16.00" | 10 | 16.00" | 23.75" | 11.25" | 16" | 48 | 6 |
| 250 WR 95 | 2.50" | 9.50" | 10 | 9.50" | 23.75" | 4.75" | 16" | 42 | 6 |
| 250 WR 11 | | 11.875" | 10 | 11.875" | 23.75" | 7.125" | 16" | 42 | 6 |
| 250 WR 14 | | 14.00" | 10 | 14.00" | 23.75" | 9.25" | 16" | 48 | 6 |
| 250 WR 16 | | 16.00" | 10 | 16.00" | 23.75" | 11.25" | 16" | 48 | 6 |
| 350 WR 95 | 3.50" | 9.50" | 10 | 9.50" | 23.75" | 4.75" | 16" | 42 | 6 |
| 350 WR 11 | | 11.875" | 10 | 11.875" | 23.75" | 7.125" | 16" | 42 | 6 |
| 350 WR 14 | | 14.00" | 10 | 14.00" | 23.75" | 9.25" | 16" | 48 | 6 |
| 350 WR 16 | | 16.00" | 10 | 6.00" | 23.75" | 11.25" | 16" | 48 | 6 |



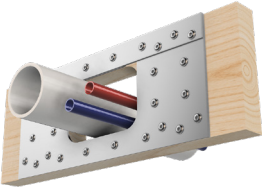
iJoist Notch Reinforcers

| Model No | Application | Gauge | Dimensions (in) | | Maximum Notch Size (in) | | Fastener Qty | No of Components |
|----------|-------------------------------------|-------|-----------------|--------|-------------------------|----|--------------|------------------|
| | | | H | W | H | W | | |
| 150 NR | 150 Flange x 9.5, 11 7/8", 14", 16" | 10 | 7.875" | 23.75" | 3.5" | 6" | 36 | 2 |
| 250 NR | 250 Flange x 9.5, 11 7/8", 14", 16" | 10 | 7.875" | 23.75" | 3.5" | 6" | 36 | 2 |
| 350 NR | 350 Flange x 9.5, 11 7/8", 14", 16" | 10 | 7.875" | 23.75" | 3.5" | 6" | 36 | 2 |

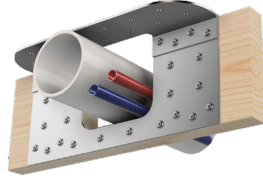


Metallurgical Information

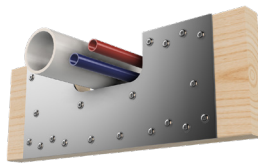
Metallurgical information below pertain to all Skyline Brands, LLC Joist Reinforcer Modular and Component products (Patents Pending).



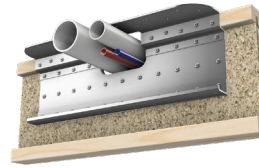
Hole Reinforcers
2x6, 2x8, 2x10



Deep Notch Reinforcers
2x6, 2x8, 2x10



Shallow Notch Reinforcers
2x8, 2x10



iJoist Flange Reinforcers
Flange widths 1.5", 2.5", 3.5"
Joist Heights 9.5", 11 7/8",
14", 16"



iJoist Web Reinforcers
Flange widths: 1.5", 2.5", 3.5"
Joist Heights: 9.5", 11 7/8",
14", 16"

| Typical Ranges of Mechanical Properties ^{A, B} (Non-mandatory) | | | | | |
|---|--------------------------|---------|---------------------------------|-----------------------------|---------------------------|
| Designation | (Longitudinal Direction) | | | | |
| | Yield Strength | | Elongation in 2 in [50mm], % | r_m Value ^C | n Value ^D |
| | ksi | [MPa] | | | |
| CS Type B | 30/55 | 205/380 | 20 | E | E |
| Fy = 30 ksi yield strength | | | | | |
| Ft = 55 ksi tensile strength | | | | | |



1945 Airport Road
Columbus, MS 39701
Phone: 662-245-4200
Fax: 662-245-4297

Metallurgical Certification



Order Number: 528429-1
 Order Dimensions: 0.1300 x 60.0000 (in) (MIN)
 Ordered Product: A653 CS Type B - 20
 PRIME GALV HOT ROLLED SHEET
 Part Number: 324329
 Alt Part#:
 Sold To:
 Ship To:
 Customer PO Number: 0101623-1
 Load Number: S1345590
 Cert Number: 2198942
 Ship Date: 2/15/2023

Chemical Analysis

| Coil Number | Coil Weight | Heat | C | Mn | P | S | Si | Cu | Sn | Ni | Cr | Mo | Al | N | V | Nb | Ti | B | Ca | C(eq) |
|-------------|-------------|---------|-----|-----|------|------|-----|-----|-------|-----|-----|------|------|-------|-------|------|------|-------|-------|-------|
| 23J702559A | 24,087 lb. | A302073 | .02 | .17 | .010 | .004 | .03 | .11 | .0070 | .05 | .06 | .010 | .036 | .0066 | <.001 | .001 | .002 | .0001 | .0017 | .082 |

Test Test 1

| Mechanical Properties | Test | | Test 1 | |
|-----------------------|---------|--------|--------------------|--------------|
| | English | Metric | English | Metric |
| Yield Strength | | | Not Reported | Not Reported |
| Tensile Strength | | | Not Reported | Not Reported |
| Elongation | | | Not Reported | Not Reported |
| N-Value | | | Not Reported | Not Reported |
| Hardness - HRBW | | | Not Reported | Not Reported |
| Direction | | | | |
| Linear Footage | | | 856 ft | 261 m |
| Gauge Length | 2 inch | | | |
| Test Standard | ASTME | | | |
| Actual gauge | | | .1325 in | 3.37 mm |
| Coating ROHS | | | RoHS CHEM TREAT | |
| Coating Weight | | | G90UL | |

We hereby certify the above is correct as contained in the records of the company. All tests performed according to ASTM standards: E8, A370, E18, E415, E1019, E646, E517 or JIS Z2241 or ISO 6892 as required. All Heats are Air Killed and Ca Treated. Melted and Manufactured in the USA in compliance with the American Iron and Steel Step Certification, Buy America Act Title 23 CFR § 635.410, and the Buy American Act Title 46 CFR § 381.2 (B) requirements.

Certified by:

Makade Arch

Certificated Date: 02/22/2023

Makade Archibald
Gal/Paint Line Metallurgical
Engineer

Engineering Properties of Structural Lumber

Douglas-Fir-Larch, Hem-Fir, Spruce-Pine-Fir

Stress-graded structural lumber is produced under two systems: visual grading and machine grading. Visual structural grading is the oldest stress grading system. It is based on the premise that the mechanical properties of lumber differ from those of clear wood because many growth characteristics of lumber affect its properties; these characteristics can be seen and judged by eye (ASTM D245). The principal growth features affecting lumber properties are the size and location of knots, sloping grain, and density.

Grading rules for lumber nominally 2 x 4 in (standard 38 x 89 mm) thick (dimension lumber) are published by grading agencies (listing and addresses are given in "National Design Specification," American Forest & Paper Association, 1992 and later). For most species, allowable properties are based on test results from full-size specimens graded by agency rules, sampled according to ASTM D2915, and tested according to ASTM D4761. Procedures for deriving allowable properties from these tests are given in ASTM D1990. Allowable properties for visually graded hardwoods and a few softwoods are derived from clear-wood data following principles given in ASTM D2555.

| Design values, (lb/in ²) | | | | | | | | |
|--------------------------------------|-------------------------|------------|------------------------------|----------------------------|--|--|-----------------------|----------------------|
| Species and Commercial Grade | Size Classification, in | Bending Fb | Tension Parallel to grain Ft | Shear parallel to grain Fv | Compression perpendicular to grain Fc.L. | Compression perpendicular to grain Fc.L. | Modulus of Elasticity | Grading Rules Agency |
| Douglas-Fir-Larch | | | | | | | | |
| Select Structural | - | 1,000 | 1,000 | 95 | 625 | 1,700 | 1,900,000 | WCLIB WWPA |
| #1 and Better | 2-4 Thick | 1,150 | 775 | 95 | 625 | 1,500 | 1,800,000 | |
| #1 | - | 1,000 | 675 | 95 | 625 | 1,450 | 1,700,000 | |
| #2 | 2 and wider | 875 | 575 | 95 | 625 | 1,300 | 1,600,000 | |
| #3 | - | 500 | 325 | 95 | 625 | 750 | 1,400,000 | |
| Stud | - | 675 | 450 | 95 | 625 | 825 | 1,400,000 | |
| Construction | 2 - 4 Thick | 1,000 | 650 | 95 | 625 | 1,600 | 1,500,000 | |
| Standard | - | 550 | 375 | 95 | 625 | 1,350 | 1,400,000 | |
| Utility | 2-4 Wide | 275 | 175 | 95 | 625 | 875 | 1,300,000 | |
| Hem-Fir | | | | | | | | |
| Select Structural | - | 1,400 | 900 | 75 | 405 | 1,500 | 1,600,000 | WCLIB WWPA |
| #1 and Better | 2-4 Thick | 1,050 | 700 | 75 | 405 | 1,350 | 1,500,000 | |
| #1 | - | 950 | 600 | 75 | 405 | 1,300 | 1,500,000 | |
| #2 | 2 and Wider | 850 | 500 | 75 | 405 | 1,250 | 1,300,000 | |
| #3 | - | 500 | 300 | 75 | 405 | 725 | 1,200,000 | |
| Stud | - | 675 | 400 | 75 | 405 | 800 | 1,200,000 | |
| Construction | 2-4 Thick | 975 | 575 | 75 | 405 | 1,500 | 1,300,000 | |
| Standard | - | 550 | 325 | 75 | 405 | 1,300 | 1,200,000 | |
| Utility | 2-4 Wide | 250 | 150 | 75 | 405 | 850 | 1,100,000 | |
| Spruce-Pine-Fir | | | | | | | | |
| Select Structural | - | 1,250 | 675 | 70 | 425 | 1,400 | 1,500,000 | NLGA |
| #1, #2 | 2-4 Thick | 875 | 425 | 70 | 425 | 110 | 1,400,000 | |
| #3 | 2 and Wider | 500 | 250 | 70 | 425 | 625 | 1,200,000 | |
| Stud | - | 675 | 325 | 70 | 425 | 675 | 1,200,000 | |
| Construction | 2-4 Thick | 975 | 475 | 70 | 425 | 1,350 | 1,300,000 | |
| Standard | - | 550 | 275 | 70 | 425 | 1,100 | 1,200,000 | |
| Utility | 2-4 Wide | 250 | 125 | 70 | 425 | 725 | 1,100,000 | |

Reference: Reproduced with permission of the American Forest and Paper Association

Fastener Technical Values

ConnexTite™ Flange Head Structural Wood Screw 1/4" x 1.5" with T25 Torx Drive (See attached TER) Fasteners details included below pertain to all Skyline Brands, LLC Reinforcer modular and component products (Patents Pending). See attached SFS TER doc for additional product data.

Dimensions and strength details

| Fastener Name | Nominal Fastener Diameter | Head | | Shank Diameter ¹ (in.) | Thread Diameter (in.) | | Nominal Bending Yield (f _y , psi) | | Allowable Fastener Strength ³ | |
|---------------|---------------------------|----------------|--------------|-----------------------------------|-----------------------|-------|--|---------|--|-------------|
| | | Diameter (in.) | Height (in.) | | Minor ² | Major | Transition Zone | Shank | Tensile (lbs) | Shear (lbs) |
| Flange Head | 1/4" | 0.552 | 0.094 | 0.173 | 0.148 | 0.244 | 201,611 | 237,010 | 970 | 485 |

For SI: 1" = 25.4 mm, 1 lbf = 4.45 N, 1 psi = 6.895 kPa.

1. Shank diameter based on manufactured thickness. Finished dimensions are larger in the coated condition due to the proprietary coatings added.
2. Minor thread diameter value is calculated as the average of the upper and lower tolerances.

Withdrawal design values/face grain applications

Allowable Withdrawal Design Values by Species (Specific Gravity) (lbs.)

| Nominal Fastener Diameter (in.) | Face Grain Applications | | |
|---------------------------------|-------------------------|----------|----------|
| | SPF (.42) | DF (.50) | SP (.55) |
| 1/4" | 95 | 135 | 220 |

1. Values are stated in lbf/in of thread engagement.
2. Values shall be adjusted by all applicable adjustment factors per NDS Section 10.3 for wood screws.
3. Fastener penetration is that threaded length embedded in the main member, including the tip.

Head pull-through design values

| Min. Side Member Thickness (in.) | Nominal Fastener Diameter (in.) | Flange Head Diameter (in.) | Counter-sunk Head Diameter (in.) | SPF (.42) | | DF (.50) | | SP (.55) | |
|----------------------------------|---------------------------------|----------------------------|----------------------------------|-----------|--------------|----------|--------------|----------|--------------|
| | | | | Flange | Counter-sunk | Flange | Counter-sunk | Flange | Counter-sunk |
| 3/4" | 1/4" | 0.552 | 0.457 | 155 | 130 | 220 | 185 | 265 | 225 |
| | 5/16" | 0.705 | 0.583 | 195 | 165 | 275 | 235 | 335 | 280 |
| 1-1/2" | 1/4" | 0.552 | 0.457 | 310 | 265 | 440 | 370 | 535 | 450 |
| | 5/16" | 0.705 | 0.583 | 390 | 330 | 550 | 465 | 670 | 565 |

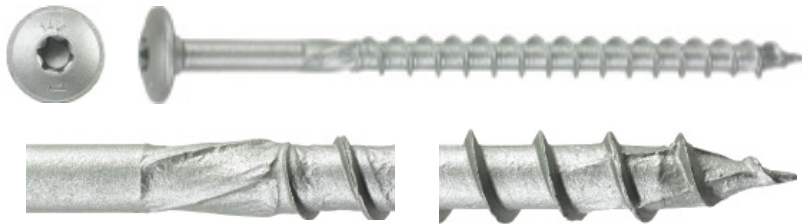
1. Values shall be adjusted by all applicable adjustment factors per NDS Section 10.3 for withdrawal of wood screws.

Lateral design values using dimensional lumber

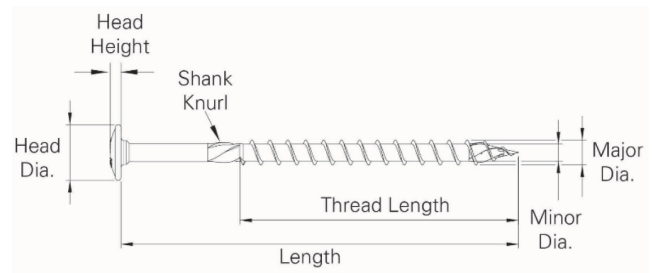
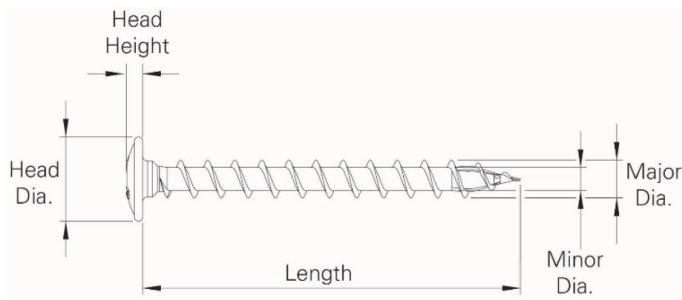
| Fastener Head Type | Nominal Fastener Diameter (in.) | Fastener Length (in.) | Side Member Thickness (in.) | Min. Penetration into Main Member (in.) | Lateral Design Values (lbs.) by Species (Specific Gravity) & Load Orientation | | | | | |
|--------------------|---------------------------------|-----------------------|-----------------------------|---|---|--------|----------|--------|----------|--------|
| | | | | | SPF (.42) | | DF (.50) | | SP (.55) | |
| | | | | | Z Para | Z Perp | Z Para | Z Perp | Z Para | Z Perp |
| Flange | 1/4" | 2-3/8" | 1-1/2" | 7/8" | 115 | 90 | 140 | 110 | 160 | 125 |
| | | 2-3/4" | | 1-1/4" | 130 | 105 | 165 | 135 | 190 | 155 |
| | | 3-1/8" | | 1-1/2" | 145 | 115 | 175 | 140 | 190 | 155 |
| | | 4" | | 2-1/2" | 145 | 115 | 175 | 140 | 190 | 155 |

1. Reference lateral design values apply to two-member single shear connections where both members are of the same specific gravity, and the fastener is oriented perpendicular to grain. Where the members are of different specific gravities, use the lower of the two.
2. Values shall be adjusted by all applicable adjustment factors per NDS.

Fastener Details



ConnexTite™ Flange Head Detail



Engineering Example

Subject: Test Results for Stiffened Floor Joist Products
 By J. Leroy Hulsey, PhD, PE, SE

Tests by a Test Lab are used for verifying calculated deformations, and calculated states of stress (flexure & shear stress).

1) Consider a timber floor joist that is stiffened using CS (commercial steel) shapes that are made to be a composite system that is a combination of both the timber floor joist and steel. The CS steel must be fastened to the timber floor joists to insure the steel will not buckle and will resist load as the timber is trying to deform.

2) The cold form steel (CFS) product satisfies current steel specifications, Code loading and NDS timber specifications.

| Material | Yield Stress (ksi) | Tensile Strength (ksi) | Grade |
|--------------------|--------------------|------------------------|-------|
| A653 CS B | 30 | 55 | G90 |
| A653 SS Grade 50/1 | 50 | 65 | G90 |
| A653 SS Grade 50/1 | 50 | 70 | G90 |
| A653 SS Grade 50/1 | 50 | 60 | G90 |
| A653 SS Grade 50/1 | 55 | 70 | G90 |
| A653 SS Grade 50/1 | 60 | 70 | G90 |
| A653 SS Grade 50/1 | 60 | 61.5 | G90 |

| Material Thickness | |
|--------------------|-----------|
| Gauge | Thickness |
| 20 | 0.0346 |
| 18 | 0.0451 |
| 16 | 0.0556 |
| 14 | 0.0712 |
| 12 | 0.1017 |
| 10 | 0.1242 |

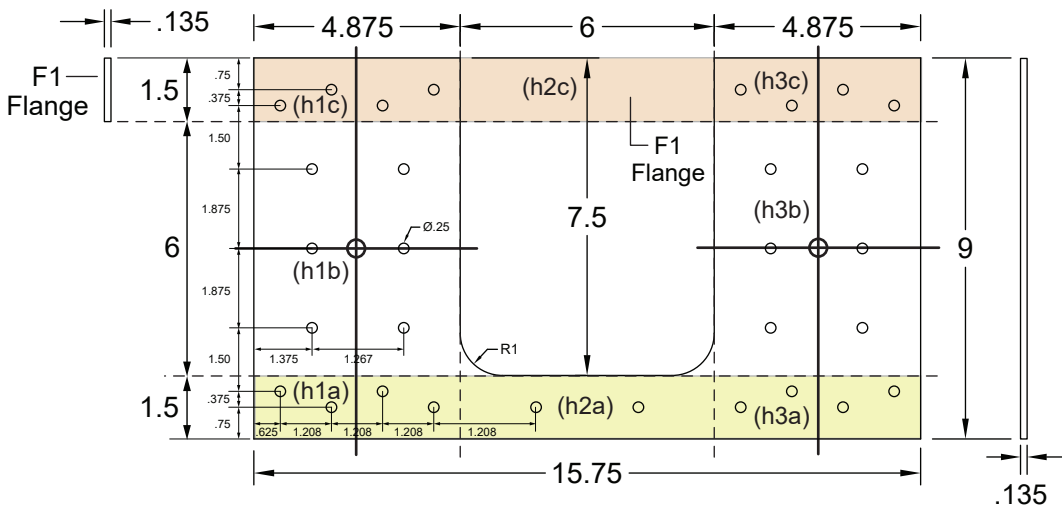
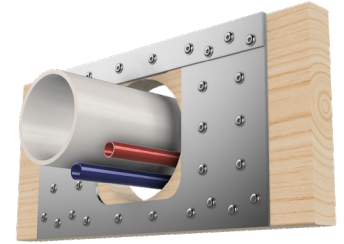
a) ASTM - American Society for Testing & Materials
 b) ASTM A653
 Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy Coated (Galvannealed) by the Hot-Dip Process 1. This standard is issued under the fixed designation A 653/A 653M; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last re-approval. A superscript epsilon (e) indicates an editorial change since the last revision or re-approval.
 (c) ASTM A653 CS B - CS Type B (also known as Commercial Steel Type B) is the most common form of commercial steel. It is designated by chemical requirements, not mechanical properties, with a slightly higher max range of carbon content (0.15%) than other types.
 (d) ASTM A653 SS (this is known as Structural Steel)
 Link: https://www.cosasteel.com/wp-content/uploads/2020/11/astm-a653_.pdf

| V1 Undamaged 2x10 (see fig 1) | | | |
|---|-------------|-------------------|--------|
| 1) Un-damaged 2x10 - #2 Douglas Fir, 2' OC with 9.5' span length | | | |
| Fbx = 0.875 ksi; Fv (parallel) = 0.095 ksi; E - 1,600 ksi | | | |
| $I = bh^3/12 = 1.5^3(9.25^3)/12 = 98.93164063 \text{ in}^4$ Sx = 21.390 cu in | | | |
| Examined using ASD (Allowable Stress Design Method): | | | |
| 3. NDS code loading is: | | | |
| a) Dead Load | | b) Live Load: | 50 psf |
| Finished floor 4 psf: | 4 psf | | |
| 3/4" sub-floor: | 3 psf | | |
| 2x10 @ 24" OC: | 6 psf | | |
| 2x10 @ 24" OC: | 6 psf | | |
| Total Dead Load = | 13 psf | Total Live Load = | 50 psf |
| c) Total DL + LL = | 63 psf | | |
| 4) Load acting on each floor joist | 126 plf | | |
| a) Moment (in-k) =(max) | | | |
| $M = 12^2 w^*(L^2)/8$ | 18.711 in-k | fb = 0.8747 ksi | (OK) |
| b) Reaction/Shear V(max) = w*L/2 | | | |
| R = V(max) = | 1.01 kips | fv = 0.0727 ksi | (OK) |
| 5) Maximum Deflection (center line) | | | |
| a) Def = 5wL4/(384EI) | | | |
| 0.1755 in | | 0.3316 L/360 | |

Engineering Example

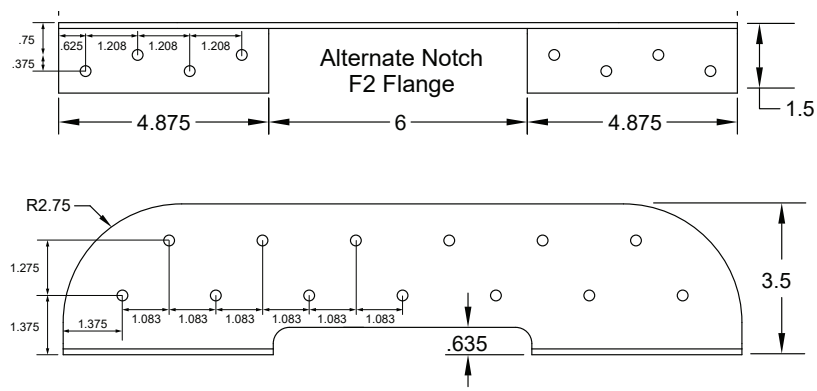
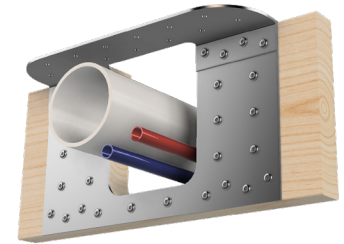
Hole Reinforcer Stiffener Plate and Flange

Fig. 1



Notch Reinforcer Stiffener Plate and Flange

Fig. 2



Engineering Example

V2 Damaged 2x10

1 Solve - Centroid of sections 1, 2 & 3 (Fig. 1 & 2)

Steel thickness = 0.135"

$I_x = bh^3/12$

| (i) | bi | A1 | yi | Aiyi | di | di ² | Aidi ² | Ix0 | Ixi |
|------------------------------------|-------|-------------------------|------|-------------|--------------|-----------------|-------------------|------------|-------------|
| 1a | 0.135 | .658125 | 0.75 | 0.49359375 | 4.636363636 | 21.49586777 | 14.14696798 | 0.03796875 | 0.03796875 |
| 1b | 0.135 | 0.81 | 4.5 | 3.645 | 0.8863636364 | 0.7856404959 | 0.6363688017 | 4.74609375 | 4.74609375 |
| 1c | .127 | 1.31625 | 0.75 | 10.8590625 | 2.863636364 | 8.200413223 | 10.7937939 | 0.0759375 | 0.0759375 |
| Sum | | 2.784375 | | 14.99765625 | | | | | 30.43713068 |
| y(bar = sum(Aiyi)/Sum(Ai) = | | 5.386363636 (in) | | | | | | | |
| di = ybar - yi = | | 4.636363636 | | | | | | | |
| (i) | bi | A1 | yi | Aiyi | di | di ² | Aidi ² | Ix0 | Ixi |
| 2a | .135 | 0.81 | 0.75 | 0.6075 | 3.75 | 14.0625 | 11.390625 | 0.03796875 | 11.42859375 |
| 2b | .135 | 0.81 | 4.5 | 3.645 | 0 | 0 | 0 | 4.74609375 | 4.74609375 |
| 2c | .135 | 0.81 | 8.25 | 6.6825 | -3.75 | 14.0625 | 11.390625 | 0.0759375 | 11.42859375 |
| Sum | | 2.43 | | 10.935 | | | | | 27.60328125 |
| y(bar = sum(Aiyi)/Sum(Ai) = | | 4.50 (in) | | | | | | | |
| di = ybar - yi = | | 3.75 | | | | | | | |
| (i) | bi | A1 | yi | Aiyi | di | di ² | Aidi ² | Ix0 | Ixi |
| 3a | .135 | 0.658125 | 0.75 | 0.49359375 | 4.636363636 | 21.49586777 | 14.14696798 | 0.03796875 | 14.18493673 |
| 3b | .135 | 0.81 | 4.5 | 3.645 | 0.8863636364 | 0.7856404959 | 0.6363688017 | 4.74609375 | 5.382462552 |
| 3c | .27 | 1.31625 | 8.25 | 10.8590625 | -2.863636364 | 8.200413223 | 10.7937939 | 0.0759375 | 10.8697314 |
| Sum | | 2.784375 | | 14.99765625 | | | | | 30.43713068 |
| y(bar = sum(Aiyi)/Sum(Ai) = | | 5.386363636 (in) | | | | | | | |
| di = ybar - yi = | | 4.636363636 | | | | | | | |

2 Solve - Convert to Douglas Fir: n=18.125

Sections 1 & 3

| | | | | | | | | | |
|------------------------------|-------------|-----------------|--|--|--|--|--|--|--|
| $t = 0.135 \times 18.125 =$ | 0.37584 | in | | | | | | | |
| $A(1) = 18.125 \times 2.784$ | 50.46 | sq in | | | | | | | |
| $I_x (in^4) =$ | 551.725 | in ⁴ | | | | | | | |
| $S_x = I_x / (9.25/2)$ | 119.2918919 | cu in | | | | | | | |

Section 2

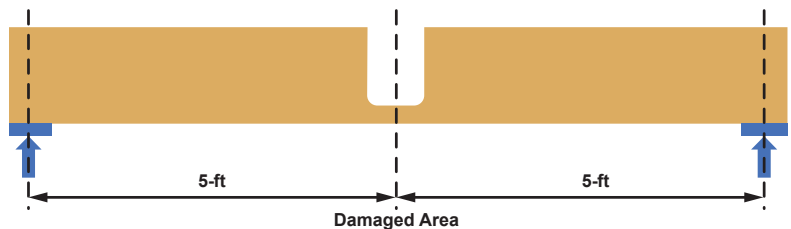
| | | | | | | | | | |
|-----------------------------|-------------|-----------------|--|--|--|--|--|--|--|
| $t = 0.135 \times 18.125 =$ | 0.37584 | inches | | | | | | | |
| $A(2) =$ | 44.04375 | sq in | | | | | | | |
| $I_x (in^4) =$ | 500.25 | in ⁴ | | | | | | | |
| $S_x = I_x / (9.25/2)$ | 216.3243243 | cu in | | | | | | | |

3 Solve Maximum Bending Stress

| | | | | | | | | | |
|----------------------------------|---|--|--|--|--|--|--|--|--|
| $M(max) = wL^2/8$ | 10.5(lbs/in) * ((9.95*12) ²)/8 in-lbs | | | | | | | | |
| $M(max) = 0.5 * (9.95*12)^2/8 =$ | 18711.4725c in-lbs | | | | | | | | |
| $f_{bx} = (18711/(S_x)) =$ | 86.625 psi | | | | | | | | |
| Fbx (allowable) = 875 psi | 875 psi | | | | | | | | |

Damaged 2x10 - Fig 3.

Limit centerline live load deflection to L/360

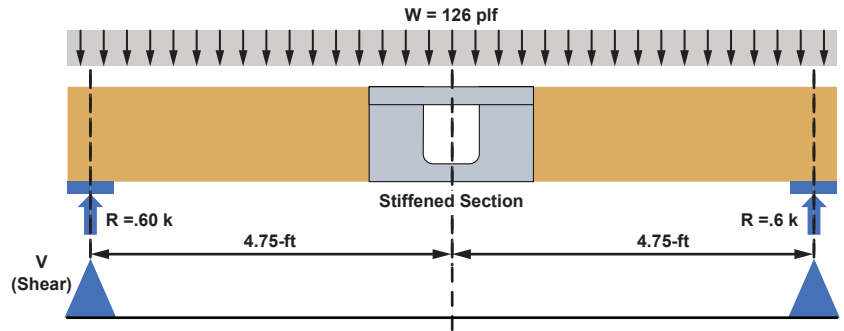


Engineering Example

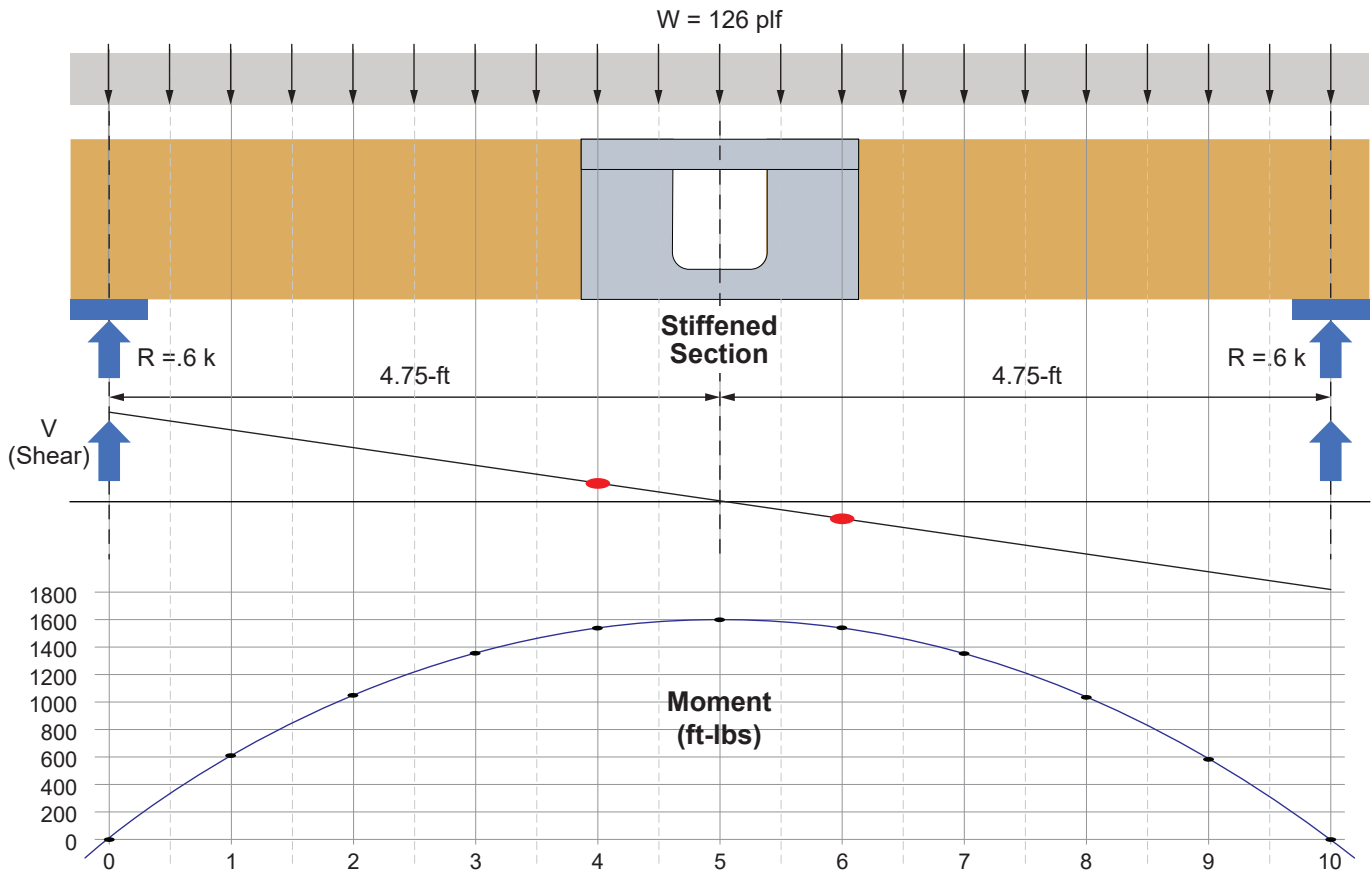
Stiffened 2x10 - Fig 4.

Note: ASTM A653 CS-B
 • Modulus of Elasticity for the Douglas Fir is 1600 ksi
 • Modulus of Elasticity for the steel plates are 29,000 ksi
 Modular ratio $Is n = Es/Ew = 29000/1600 = 18.125$

Converting to Douglas Fir:
 Stiffened section will be: $ts = 0.135 \times 18.125 = 2.45$ inches.
 Add the Stiffened to existing douglas fir section = 1.5" thick
 the Stiffened section becomes: $t = 2 \times 2.45 + 1.5 = 6.4$ " thick.
 Stiffened flange section + web becomes: $tf = 11.3$ " thick.

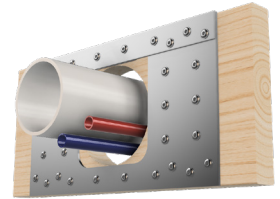


Title - Fig 5.



It is the purpose of this example to show the reader how to improve the performance for a damaged 2x10 floor joist. In this case, damage is at mid-span. A 10 gauge CS-B metal stiffener is installed at mid-span to strength the floor joist. This is a combination of a #2 grade DF 2x10 and a steel stiffener. Bending stress for both wood and steel are check to be within allowable. In addition, the centerline deflection is kept within allowable live load deflection. - Dr. J. Leroy Hulsey, Ph.D., P. E., S.E.

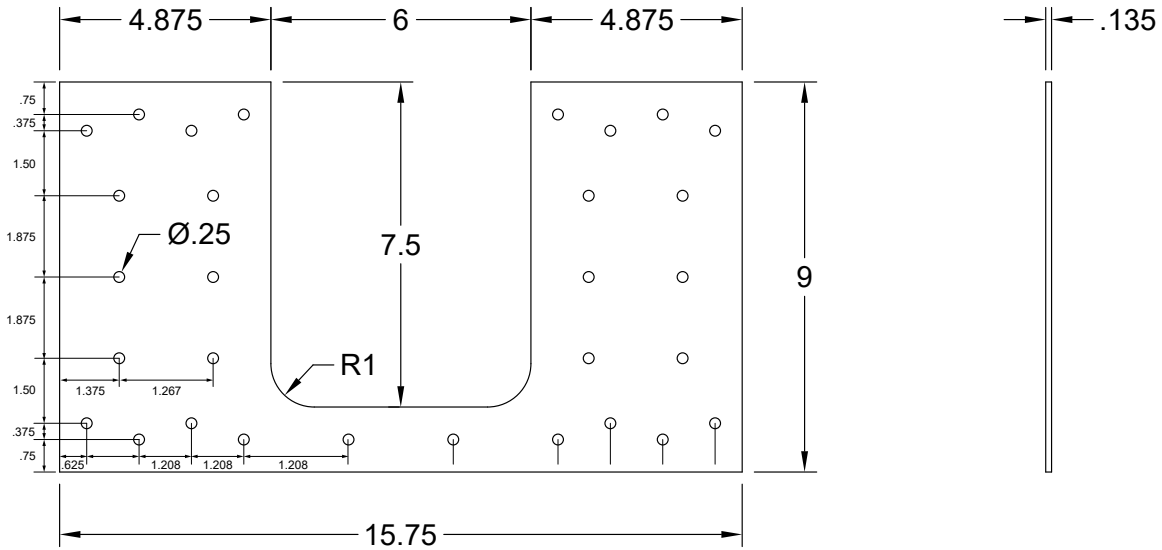




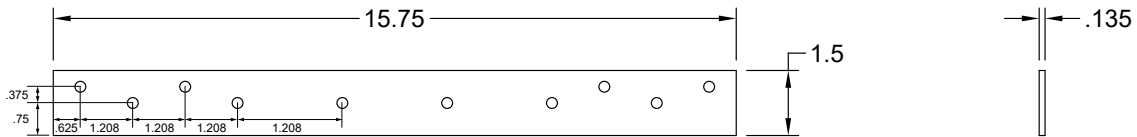
Technical Drawings

2x10 HR Hole Reinforcer (Patents Pending)

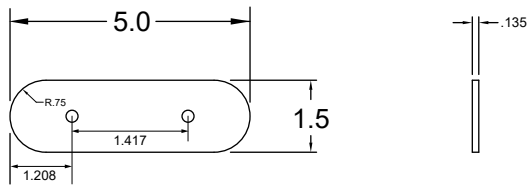
Component 1 : 2x10 HNR Web Plate



Component 2: F1 Flange

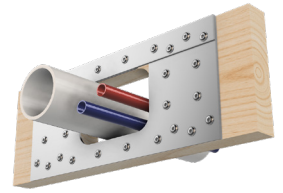


Component 3: S1 Spacer

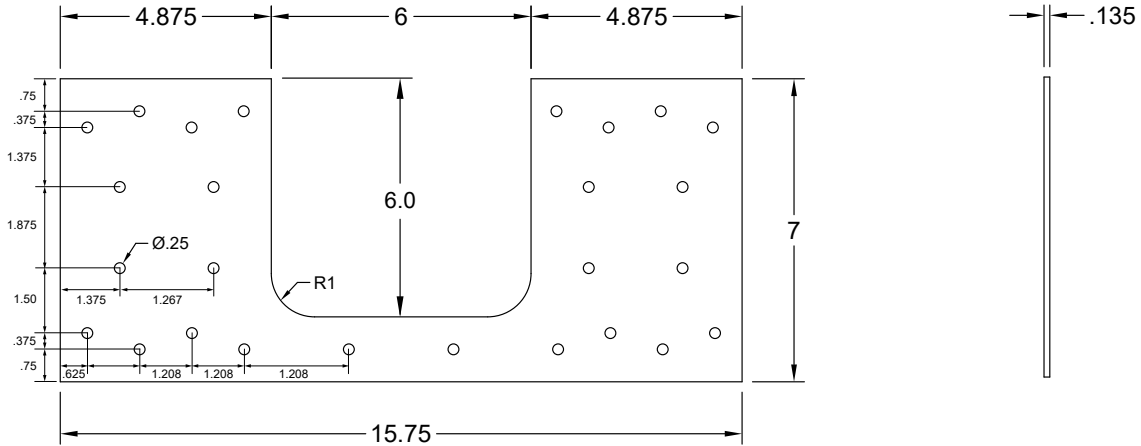


Technical Drawings

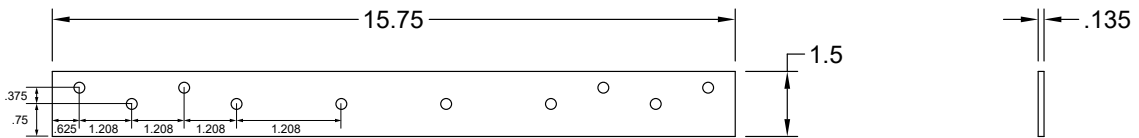
2x8 HR Hole Reinforcer (Patents Pending)



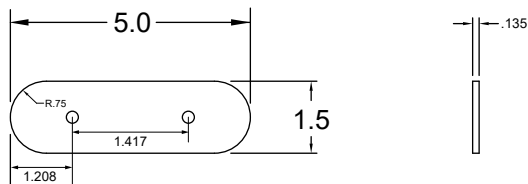
Component 1: 2x8 HNR Web Plate

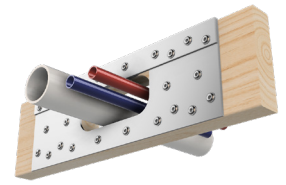


Component 2: F1 Flange



Component 3: S1 Spacer

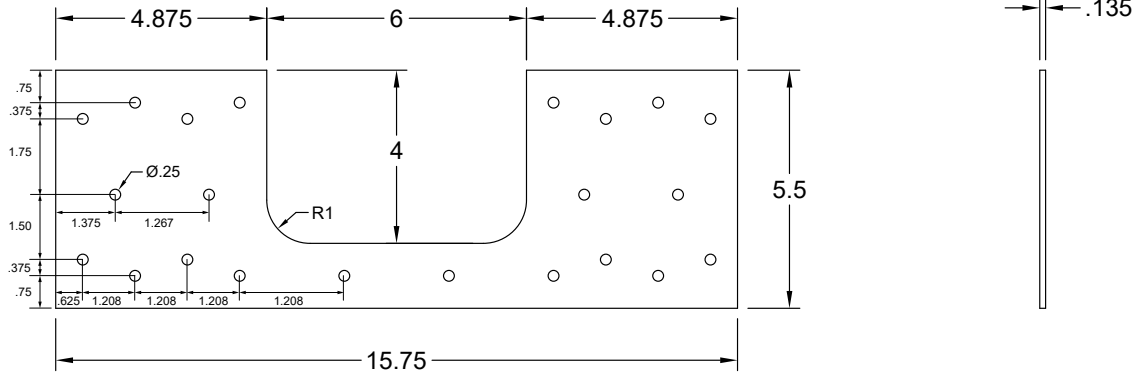




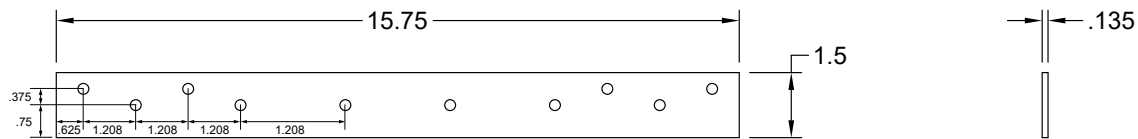
Technical Drawings

2x6 HR Hole Reinforcer (Patents Pending)

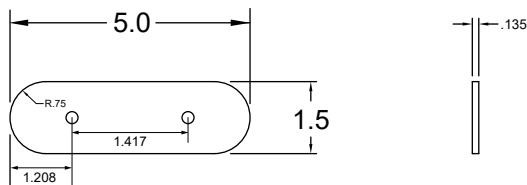
Component 1: 2x6 HNR Web Plate

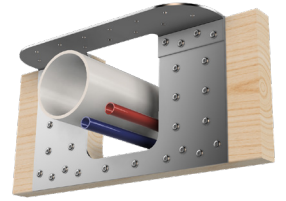


Component 2: F1 Flange



Component 2: S1 Spacer

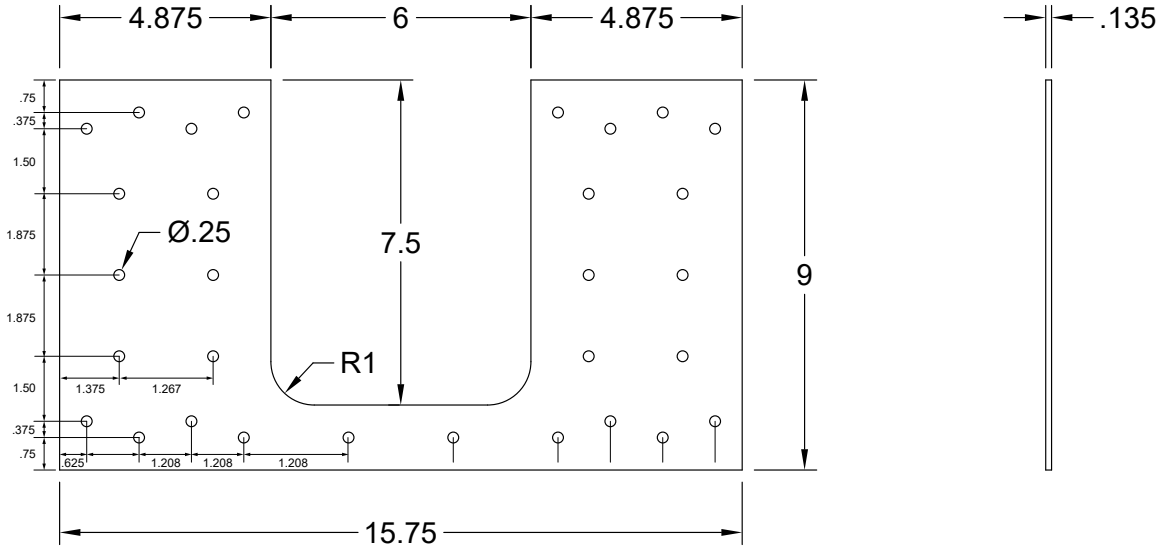




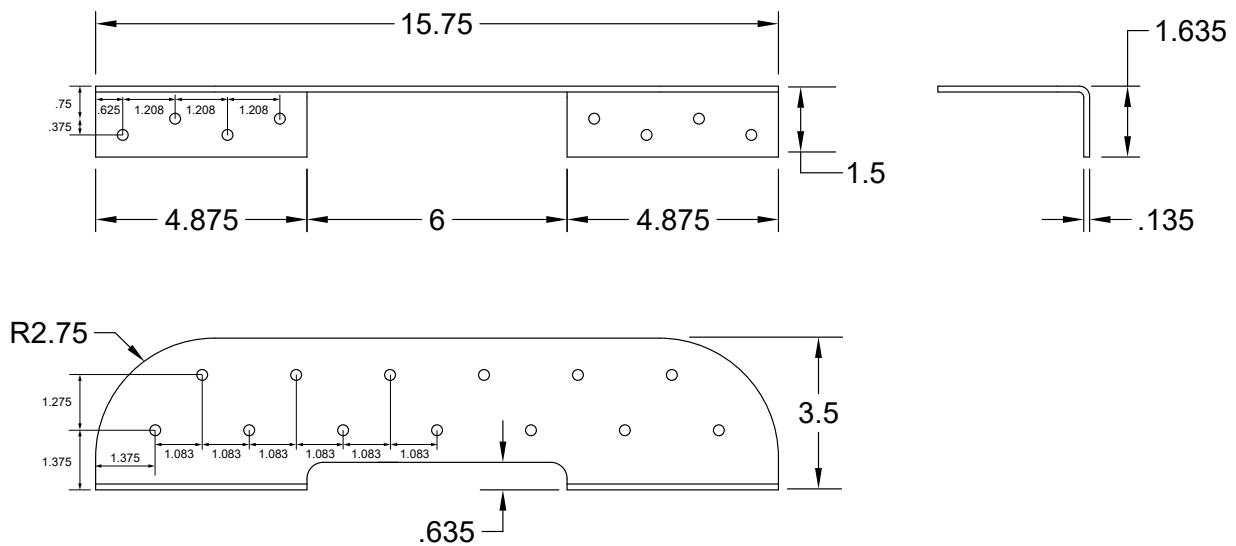
Technical Drawings

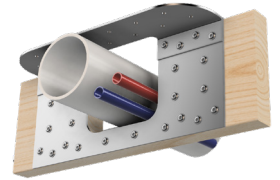
2x10 DNR Deep Notch Reinforcer (Patents Pending)

Component 1: 2x10 HNR Web Plate



Component 2: F2 Notch Flange

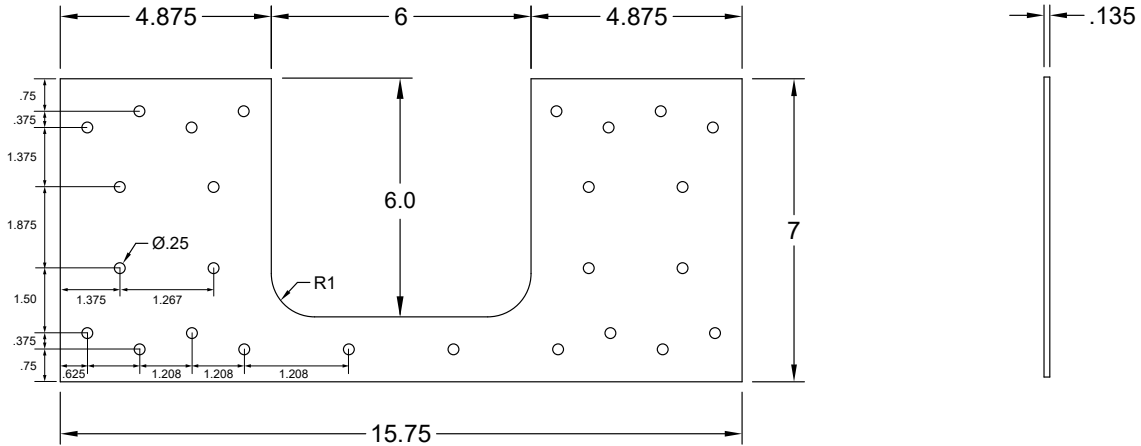




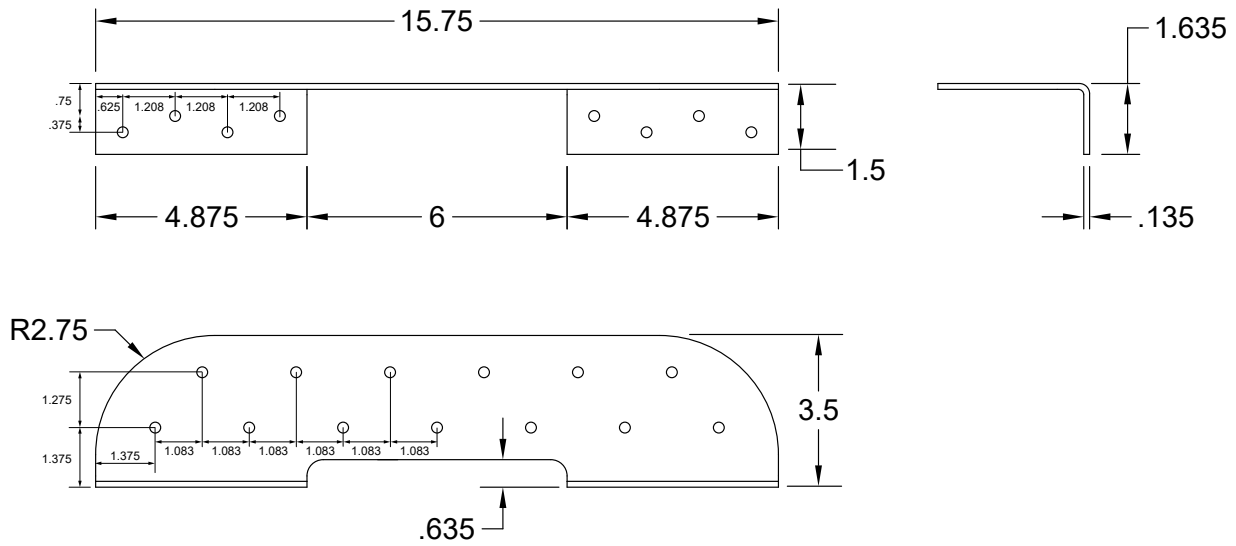
Technical Drawings

2x8 DNR Deep Notch Reinforcer (Patents Pending)

Component 1: 2x8 HNR Web Plate

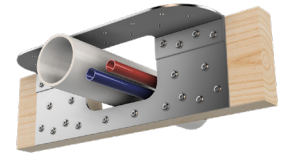


Component 2: F2 Notch Flange

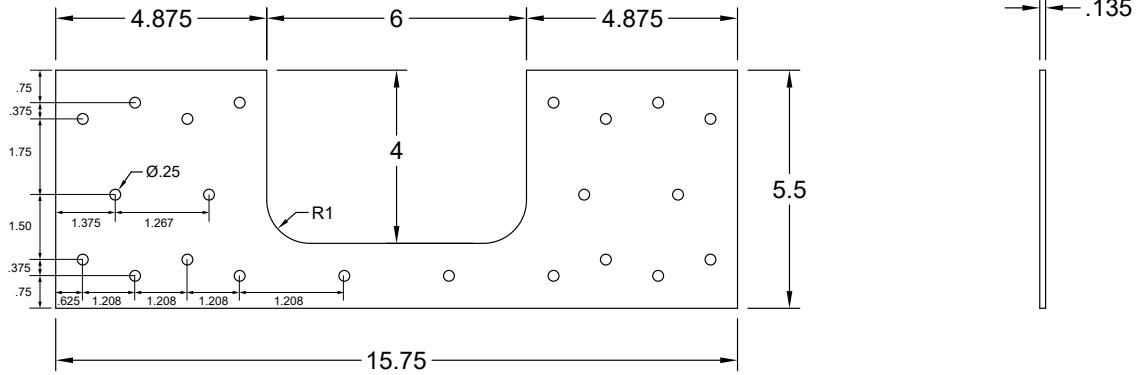


Technical Drawings

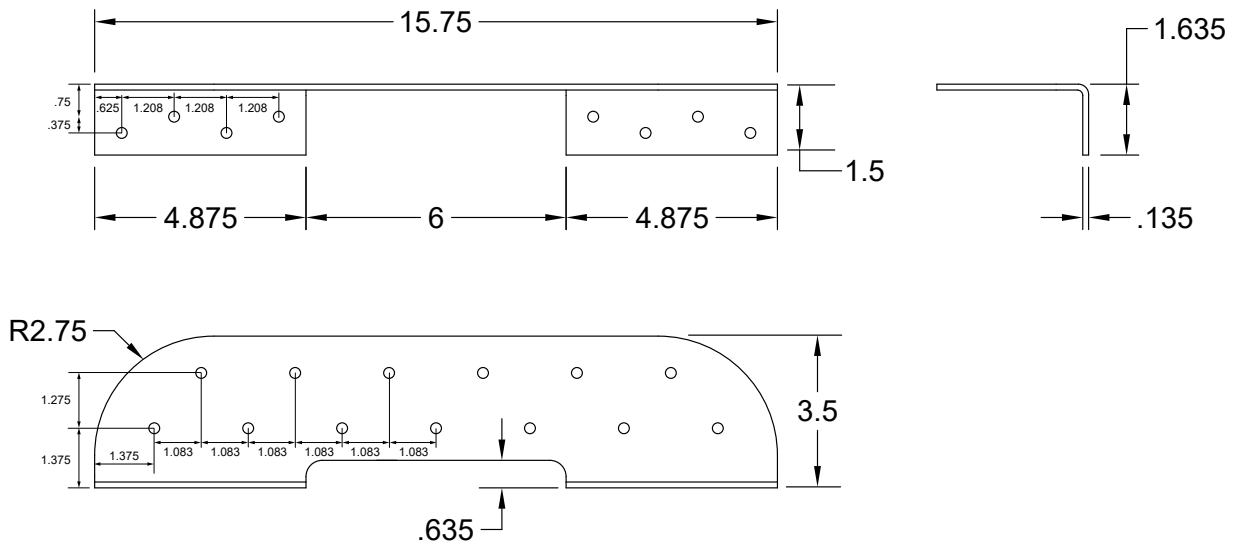
2x6 DNR Deep Notch Reinforcer (Patents Pending)

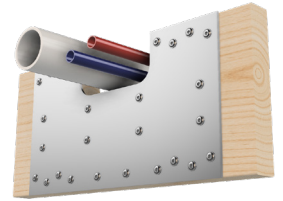


Component 1: 2x6 HNR Web Plate



Component 2: F2 Notch Flange

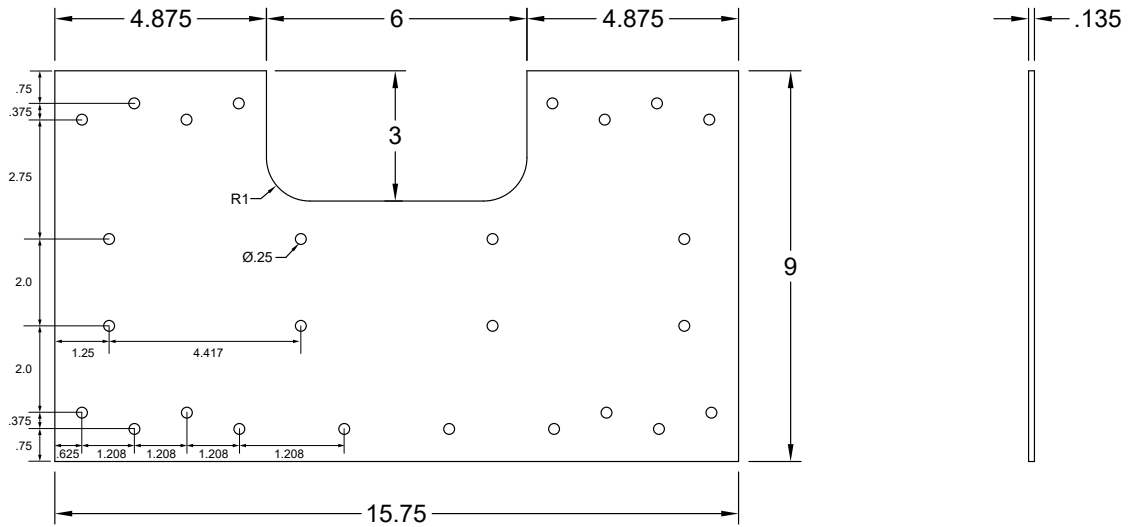




Technical Drawings

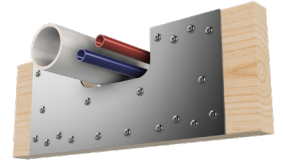
2x10 SNR Shallow Notch Reinforcer (Patents Pending)

Component 1: 2x10 SNR Web Plate



Technical Drawings

2x8 SNR Shallow Notch Reinforcer (Patents Pending)



Component 1: 2x8 SNR Web Plate

