ANCHO



POWER-STUD®+ SD4/SD6

Stainless Steel Wedge Expansion Anchors

PRODUCT DESCRIPTION

The Power-Stud+ SD4 and Power-Stud+ SD6 anchors are fully threaded, torque-controlled, stainless steel wedge expansion anchors which are designed for consistent performance in cracked and uncracked concrete. Suitable base materials are normal-weight, sand-lightweight concrete, and grouted concrete masonry (CMU). The anchor is manufactured with a stainless steel body and expansion clip. Nut and washer are included.

GENERAL APPLICATIONS AND USE

- Structural connections, i.e., beam and column anchorage
- Safety-related and common attachments
- Interior and exterior applications
- Tension zone applications, i.e., cable trays and strut, pipe supports, fire sprinklers

FEATURES AND BENEFITS

- + Knurled mandrel design provides consistent performance in cracked concrete and helps prevent galling during service life.
- + Nominal drill bit size is the same as the anchor diameter
- + Anchor can be installed through standard clearance fixture holes
- + Length ID code and identifying marking stamped on head of each anchor
- + Anchor design allows for follow-up expansion after setting under tensile loading
- + Corrosion resistant stainless steel anchors
- + Domestically manufactured by request

APPROVALS AND LISTINGS

- International Code Council Evaluation Service (ICC-ES), ESR-2502 for cracked and uncracked concrete
- Code compliant with the 2015 IBC, 2015 IRC, 2012 IBC, 2012 IRC, 2009 IBC, and 2009 IRC
- Tested in accordance with ACI 355.2/ASTM E 488 and ICC-ES AC193 for use in structural concrete under the design provisions of ACI 318-14 Chapter 17 or ACI 318-11/08 Appendix D
- Evaluated and qualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading (Category 1 anchors)

GUIDE SPECIFICATIONS

CSI Divisions: 03 16 00-Concrete Anchors, 04 05 19.16 - Masonry Anchors and 05 05 19 Post-Installed Concrete Anchors. Expansion anchors shall be Power-Stud+ SD4 and Power-Stud+ SD6 as supplied by DEWALT, Towson, MD. Anchors shall be installed in accordance with published instructions and the Authority Having Jurisdiction.

MATERIAL SPECIFICATIONS

Anchor component	Specif	Specification						
Anchor component	SD4 ^{1,}	SD6 ¹						
Anchor body	Type 304 Stainless Steel	Type 316 Stainless Steel						
Washer	300 Series Stainless Steel Type 316 Stainless Steel							
Hex Nut	Type 316 Stainless Steel							
Expansion wedge (clip) Type 316 Stainless Steel								
1. Domestically manufactured anchors are a	vailable upon request (made to order, see orde	ring information for details).						

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POWER-STUD+ STAINLESS STEEL ASSEMBLY

THREAD VERSION

· UNC threaded stud

ANCHOR MATERIALS

• Stainless steel body and expansion clip, nut and washer

ANCHOR SIZE RANGE (TYP.)

• 1/4" diameter through 3/4" diameter

SUITABLE BASE MATERIALS

- Normal-weight concrete
- Sand-lightweight concrete
- Grouted Concrete Masonry (CMU)





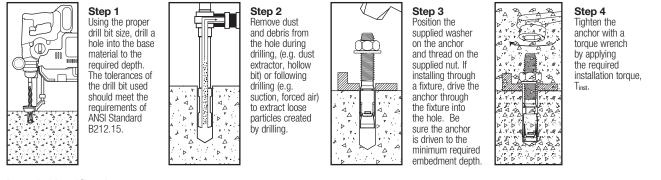






INSTALLATION INSTRUCTIONS



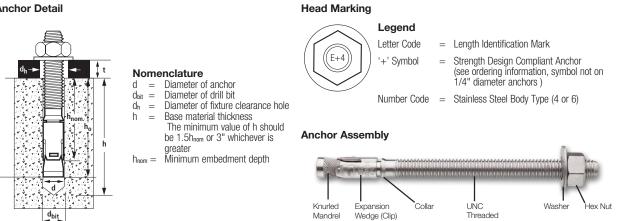


Length Identification

Mark A B C From 1-1/2" 2" 2-1/2" Up to	D 3" 3-	E F -1/2" 4"	G 4-1/2"	H	I	J	К	L	м	N	0	Р	Q	R
	3" 3-	-1/2" 4"	4 1/0"											
Up to		17 - 1	4-1/Z	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"	9-1/2"	10"
	3-1/2"	4" 4-1/2"	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"	9-1/2"	10"	11"

Length identification mark indicates overall length of anchor

Anchor Detail



Stud

REFERENCE DATA (ASD)

Installation Specifications Table for Power-Stud+ SD4 and Power-Stud+ SD6 in Concrete

Anchor Property/Setting Information	Notation	Units		Nomin	Nominal Anchor Diameter (inch)				
Anchor Property/setting information	Notation	Units .	1/4	3/8	1/2	5/8	3/4		
Anchor outside diameter	d	in. (mm)	0.250 (6.4)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)		
Thread Size (UNC)	-	in.	1/4-20	3/8-16	1/2-13	5/8-11	3/4-10		
Nominal drill bit diameter	d _{bit}	in.	1/4 ANSI	3/8 ANSI	1/2 ANSI	5/8 ANSI	3/4 ANSI		
Minimum diameter of hole clearance in fixture	dh	in. (mm)	5/16 (7.9)	7/16 (11.1)	9/16 (14.3)	11/16 (17.5)	13/16 (20.6)		
Minimum embedment depth	h _{nom}	in. (mm)	1-1/8 (29)	1-3/8 (41)	1-7/8 (48)	2-1/2 (64)	3-3/8 (86)		
Minimum hole depth	h₀	in. (mm)	1-1/4 (32)	1-1/2 (38)	2 (51)	2-5/8 (67)	3-1/2 (89)		
Installation torque	Tinst	ftlbf. (N-m)	6 (8)	25 (34)	40 (54)	60 (81)	110 (149)		
Torque wrench/socket size	-	in.	7/16	9/16	3/4	15/16	1-1/8		
Nut height	-	in.	7/32	21/64	7/16	35/64	41/64		

HANICAL ANCHORS

Stainless Steel Wedge Expansion Anchors SD4/SD6 POWER-STUD®+



MECHANICAL ANCHORS

POWER-STUD®+ SD4/SD6 Stainless Steel Wedge Expansion Anchors

					Minim	um Concrete C	ompressive S	trength			
Nominal Anchor	Minimum Embedment Depth	f ^ı c = 2, (17.3	,500 psi MPa)	f ⁱ c = 3, (20.7	000 psi MPa)		000 psi MPa)	f'c = 6, (41.4		f'c = 8, (55.2	
Diameter in.	hnom in. (mm)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Sheai Ibs (kN)
1/4	1-1/8	1,095	2,135	1,200	2,135	1,390	2,135	1,455	2,135	1,680	2,135
	(29)	(4.9)	(9.5)	(5.3)	(9.5)	(6.2)	(9.5)	(6.5)	(9.5)	(7.5)	(9.5)
1/4	1-3/4	1,890	2,135	2,070	2,135	2,390	2,135	2,480	2,135	2,480	2,135
	(44)	(8.4)	(9.5)	(9.2)	(9.5)	(10.6)	(9.5)	(11.0)	(9.5)	(11.0)	(9.5)
_	1-3/8	1,530	2,745	1,680	2,745	1,940	2,745	2,520	2,745	2,910	2,745
	(41)	(6.8)	(12.2)	(7.5)	(12.2)	(8.6)	(12.2)	(11.2)	(12.2)	(12.9)	(12.2)
3/8	1-7/8	2,790	2,745	3,060	2,745	3,530	2,745	4,195	2,745	4,840	2,745
	(48)	(12.4)	(12.2)	(13.6)	(12.2)	(15.7)	(12.2)	(18.7)	(12.2)	(21.5)	(12.2
	3	4,700	2,745	4,895	2,745	4,895	2,745	4,895	2,745	4,895	2,745
	(76)	(20.9)	(12.2)	(21.8)	(12.2)	(21.8)	(12.2)	(21.8)	(12.2)	(21.8)	(12.2
	1-7/8	2,745	5,090	3,010	5,090	3,475	5,090	4,525	5,090	5,230	5,090
	(48)	(12.2)	(22.6)	(13.4)	(22.6)	(15.5)	(22.6)	(20.1)	(22.6)	(23.3)	(22.6
1/2	2-3/8	5,370	5,090	5,880	5,090	6,790	5,090	6,790	5,090	7,845	5,090
	(60)	(23.9)	(22.6)	(26.2)	(22.6)	(30.2)	(22.6)	(30.2)	(22.6)	(34.9)	(22.6
	3-3/4	8,840	5,090	9,300	5,090	9,300	5,090	9,300	5,090	9,300	5,090
	(95)	(39.3)	(22.6)	(41.4)	(22.6)	(41.4)	(22.6)	(41.4)	(22.6)	(41.4)	(22.6
	2-1/2	5,015	9,230	5,495	9,230	6,345	9,230	7,250	9,230	8,370	9,230
	(64)	(22.3)	(41.1)	(24.4)	(41.1)	(28.2)	(41.1)	(32.2)	(41.1)	(37.2)	(41.1
5/8	3-1/4	6,760	9,230	7,405	9,230	8,560	9,230	9,615	9,230	11,105	9,230
	(83)	(30.1)	(41.1)	(32.9)	(41.1)	(38.1)	(41.1)	(42.8)	(41.1)	(49.4)	(41.1
	4-3/4	10,550	9,230	11,555	9,230	13,345	9,230	14,560	9,230	14,560	9,23
	(121)	(46.9)	(41.1)	(51.4)	(41.1)	(59.4)	(41.1)	(64.8)	(41.1)	(64.8)	(41.1
	3-3/8	6,695	11,255	7,330	12,625	8,465	14,580	9,705	15,440	11,210	15,44
	(86)	(29.8)	(50.1)	(32.6)	(56.2)	(37.7)	(64.9)	(43.2)	(68.7)	(49.9)	(68.7
3/4	4-1/2	10,800	15,440	11,830	15,440	13,575	15,440	17,110	15,440	19,760	15,44
	(114)	(48.0)	(68.7)	(52.6)	(68.7)	(60.4)	(68.7)	(76.1)	(68.7)	(87.9)	(68.7
	5-5/8	11,730	15,440	12,850	15,440	13,575	15,440	19,710	15,440	21,705	15,44
	(143)	(52.2)	(68.7)	(57.2)	(68.7)	(60.4)	(68.7)	(87.7)	(68.7)	(96.5)	(68.7

time of installation.

2. Ultimate load capacities must be reduced by a minimum safety factor of 4.0 or greater to determine allowable working loads.

Allowable Load Capacities for Power-Stud+ SD4 and Power-Stud+ SD6 in Normal-Weight Concrete 1,2,3,4

					Minim	um Concrete C	ompressive St	rength			
Nominal Anchor	Minimum Embedment Depth		500 psi MPa)	f ⁱ c = 3, (20.7		f ^ı c = 4, (27.6		f ^ı c = 6, (41.4			,000 psi MPa)
Diameter in.	h _{nom} in. (mm)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)	Tension Ibs (kN)	Shear Ibs (kN)
1/4	1-1/8	275	535	300	535	350	535	365	535	420	535
	(28)	(1.2)	(2.4)	(1.3)	(2.4)	(1.6)	(2.4)	(1.6)	(2.4)	(1.9)	(2.4)
1/4	1-3/4	475	535	520	535	600	535	620	535	620	535
	(44)	(2.1)	(2.4)	(2.3)	(2.4)	(2.7)	(2.4)	(2.8)	(2.4)	(2.8)	(2.4)
	1-3/8	385	685	420	685	485	685	630	685	730	685
	(41)	(1.7)	(3.0)	(1.9)	(3.0)	(2.2)	(3.0)	(2.8)	(3.0)	(3.2)	(3.0)
3/8	1-7/8	700	685	765	685	885	685	1,050	685	1,210	685
	(60)	(3.1)	(3.0)	(3.4)	(3.0)	(3.9)	(3.0)	(4.7)	(3.0)	(5.4)	(3.0)
	3	1,175	685	1,225	685	1,225	685	1,225	685	1,225	685
	(60)	(5.2)	(3.0)	(5.4)	(3.0)	(5.4)	(3.0)	(5.4)	(3.0)	(5.4)	(3.0)
	1-7/8	685	1,275	755	1,275	870	1,275	1,130	1,275	1,310	1,275
	(57)	(3.0)	(5.7)	(3.4)	(5.7)	(3.9)	(5.7)	(5.0)	(5.7)	(5.8)	(5.7)
1/2	2-3/8	1,345	1,275	1,470	1,275	1,700	1,275	1,700	1,275	1,960	1,275
	(64)	(6.0)	(5.7)	(6.5)	(5.7)	(7.6)	(5.7)	(7.6)	(5.7)	(8.7)	(5.7)
	3-3/4	2,210	1,275	2,325	1,275	2,325	1,275	2,325	1,275	2,325	1,275
	(95)	(9.8)	(5.7)	(10.3)	(5.7)	(10.3)	(5.7)	(10.3)	(5.7)	(10.3)	(5.7)
	2-1/2	1,255	2,310	1,375	2,310	1,585	2,310	1,815	2,310	2,095	2,310
	(70)	(5.6)	(10.3)	(6.1)	(10.3)	(7.1)	(10.3)	(8.1)	(10.3)	(9.3)	(10.3)
5/8	3-1/4	1,690	2,310	1,850	2,310	2,140	2,310	2,405	2,310	2,775	2,310
	(86)	(7.5)	(10.3)	(8.2)	(10.3)	(9.5)	(10.3)	(10.7)	(10.3)	(12.3)	(10.3)
	4-3/4	2,640	2,310	2,890	2,310	3,335	2,310	3,640	2,310	3,640	2,310
	(117)	(11.7)	(10.3)	(12.9)	(10.3)	(14.8)	(10.3)	(16.2)	(10.3)	(16.2)	(10.3)
	3-3/8	1,675	2,815	1,835	3,155	2,115	3,645	2,425	3,860	2,805	3,860
	(86)	(7.5)	(12.5)	(8.2)	(14.0)	(9.4)	(16.2)	(10.8)	(17.2)	(12.5)	(17.2)
3/4	4-1/2	2,700	3,860	2,960	3,860	3,395	3,860	4,280	3,860	4,940	3,860
	(114)	(12.0)	(17.2)	(13.2)	(17.2)	(15.1)	(17.2)	(19.0)	(17.2)	(22.0)	(17.2)
	5-5/8	2,935	3,860	3,215	3,860	3,395	3,860	4,930	3,860	5,425	3,860
	(143)	(13.1)	(17.2)	(14.3)	(17.2)	(15.1)	(17.2)	(21.9)	(17.2)	(24.1)	(17.2)

1. Tabulated load values are for anchors installed in uncracked concrete. Concrete compressive strength must be at the specified minimum at the time of installation.

2. Allowable load capacities listed are calculated using and applied safety factor of 4.0.

3. Allowable load capacities must be multiplied by reduction factors when anchor spacing or edge distances are less than critical distances.

4. Linear interpolation may be used to determine allowable loads for intermediate embedments and compressive strengths.

POWER-STUD®+ SD4/SD6 Stainless Steel Wedge Expansion Anchors

SPACING DISTANCE AND EDGE DISTANCE ADJUSTMENT FACTORS FOR NORMAL WEIGHT CONCRETE - TENSION (Fns, Fnc)

Spacing Reduction Factors - Tension (F_{NS})

opuo	ny neutron ra	-				
	Diameter (in)	1/4	3/8	1/2	5/8	3/4
	inal Embed. hnom (in)	1-3/4	1-7/8	2-1/2	3-1/4	4-1/2
Minin	num Spacing, smin (in)	2	3	3	5	5
	1-3/4	-	-	-	-	-
	2	0.79	-	-	-	-
	2-1/4	0.81	-	-	-	-
	2-1/2	0.83	-	-	-	-
	2-3/4	0.85	-	-	-	-
	3	0.87	0.87	0.82	-	-
	3-1/2	0.91	0.91	0.85	-	-
	4	0.96	0.96	0.88	-	-
s)	4-1/2	1.00	1.00	0.91	-	-
che	5	1.00	1.00	0.94	0.85	0.76
Spacing Distance (inches)	5-1/2	1.00	1.00	0.97	0.87	0.78
ance	6	1.00	1.00	1.00	0.90	0.80
Dist	6-1/2	1.00	1.00	1.00	0.92	0.82
Bu	7	1.00	1.00	1.00	0.94	0.84
Jaci	7-1/2	1.00	1.00	1.00	0.97	0.86
S	8	1.00	1.00	1.00	0.99	0.87
	8-1/4	1.00	1.00	1.00	1.00	0.88
	8-1/2	1.00	1.00	1.00	1.00	0.89
	9	1.00	1.00	1.00	1.00	0.91
	9-1/2	1.00	1.00	1.00	1.00	0.93
	10	1.00	1.00	1.00	1.00	0.95
	10-1/2	1.00	1.00	1.00	1.00	0.97
	11	1.00	1.00	1.00	1.00	0.99
	11-1/4	1.00	1.00	1.00	1.00	1.00

Edge	Distance Reduct	ion Fac	tors-	Tensio	n (F _{NC})	
	Diameter (in)	1/4	3/8	1/2	5/8	3/4
Nom	inal Embed. hoom (in)	1-3/4	1-7/8	2-1/2	3-1/4	4-1/2
Critical	Edge Distance, Cac (in)	5	5	7-1/2	9-1/2	9
Min. E	dge Distance, cmin (in)	1-3/4	3	3	4-1/2	5
	1-1/2	-	-	-	-	-
	1-3/4	0.35	-	-	-	-
	2	0.40	-	-	-	-
	2-1/4	0.45	-	-	-	-
	2-1/2	0.50	-	-	-	-
	2-3/4	0.55	-	-	-	-
~	3	0.60	0.60	0.40	-	-
Edge Distance (inches)	3-1/2	0.70	0.70	0.47	-	-
(inc	4	0.80	0.80	0.53	-	-
nce	4-1/2	0.90	0.90	0.60	0.47	-
istaı	5	1.00	1.00	0.67	0.53	0.56
e Di	5-1/2	1.00	1.00	0.73	0.58	0.61
Edg	6	1.00	1.00	0.80	0.63	0.67
	6-1/2	1.00	1.00	0.87	0.68	0.72
	7	1.00	1.00	0.93	0.74	0.78
	7-1/2	1.00	1.00	1.00	0.79	0.83
	8	1.00	1.00	1.00	0.84	0.89
	8-1/2	1.00	1.00	1.00	0.89	0.94
	9	1.00	1.00	1.00	0.95	1.00
	9-1/2	1.00	1.00	1.00	1.00	1.00

SPACING DISTANCE AND EDGE DISTANCE ADJUSTMENT FACTORS FOR NORMAL WEIGHT CONCRETE - SHEAR (Fvs, Fvc)

Spacing Reduction Factors - Shear (Fvs)

Euge D			<u> </u>			IIY NEUUCUUII FA	
D	3/4	5/8	1/2	3/8	1/4	Diameter (in)	
Nomina	4-1/2	3-1/4	2-1/2	1-7/8	1-3/4	inal Embed. hnom (in)	Nom
Min. Edg	5	5	3	3	2	num Spacing, smin (in)	Minim
	-	-	-	-	-	1-3/4	
	-	-	-	-	0.87	2	
	-	-	-	-	0.88	2-1/4	
	-	-	-	-	0.90	2-1/2	
	-	-	-	-	0.91	2-3/4	
	-	-	0.89	0.92	0.92	3	
	-	-	0.91	0.95	0.95	3-1/2	
	-	-	0.93	0.97	0.97	4	
	-	-	0.95	1.00	1.00	4-1/2	(s
s)	0.84	0.91	0.96	1.00	1.00	5	che
che	0.85	0.93	0.98	1.00	1.00	5-1/2	(in
e (ji	0.86	0.94	1.00	1.00	1.00	6	ance
anci	0.88	0.95	1.00	1.00	1.00	6-1/2	listá
Edge Distance (inches)	0.89	0.97	1.00	1.00	1.00	7	Spacing Distance (inches)
ge	0.90	0.98	1.00	1.00	1.00	7-1/2	paci
Ed	0.92	0.99	1.00	1.00	1.00	8	S
	0.92	1.00	1.00	1.00	1.00	8-1/4	
	0.93	1.00	1.00	1.00	1.00	8-1/2	
	0.94	1.00	1.00	1.00	1.00	9	
	0.95	1.00	1.00	1.00	1.00	9-1/2	
	0.97	1.00	1.00	1.00	1.00	10	
	0.98	1.00	1.00	1.00	1.00	10-1/2	
	0.99	1.00	1.00	1.00	1.00	11	
	1.00	1.00	1.00	1.00	1.00	11-1/4	

	Diameter (in)	1/4	3/8	1/2	5/8	3/4
Nom	inal Embed. hnom (in)	1-3/4	1-7/8	2-1/2	3-1/4	4-1/2
Min. E	dge Distance, cmm (in)	1-3/4	3	3	4-1/2	5
	1-1/2	-	-	-	-	-
	1-3/4	0.39	-	-	-	-
	2	0.44	-	-	-	-
	2-1/4	0.50	-	-	-	-
	2-1/2	0.56	-	-	-	-
	2-3/4	0.61	-	-	-	-
	3	0.67	0.67	0.50	-	-
	3-1/2	0.78	0.78	0.58	-	-
	4	0.89	0.89	0.67	-	-
(s	4-1/2	1.00	1.00	0.75	0.55	-
Edge Distance (inches)	5	1.00	1.00	0.83	0.61	0.44
e (in	5-1/2	1.00	1.00	0.92	0.67	0.49
anc	6	1.00	1.00	1.00	0.73	0.53
Dist	6-1/2	1.00	1.00	1.00	0.79	0.58
lge	7	1.00	1.00	1.00	0.85	0.62
B	7-1/2	1.00	1.00	1.00	0.91	0.67
	8	1.00	1.00	1.00	0.97	0.71
	8-1/4	1.00	1.00	1.00	1.00	0.73
	8-1/2	1.00	1.00	1.00	1.00	0.76
	9	1.00	1.00	1.00	1.00	0.80
	9-1/2	1.00	1.00	1.00	1.00	0.84
	10	1.00	1.00	1.00	1.00	0.89
	10-1/2	1.00	1.00	1.00	1.00	0.93
	11	1.00	1.00	1.00	1.00	0.98
	11-1/4	1.00	1.00	1.00	1.00	1.00



▶ E '.'/^

PERFORMANCE DATA

Ultimate Load Capacities for Power-Stud+ SD4 and Power-Stud+ SD6 installed into the Face of Grout Filled Concrete Masonry^{1,2}

Minimum Embedment hnom in. (mm)	Minimum Edge Distance in. (mm)	Minimum End Distance in. (mm)	Ultimate Tension Load Ib (KN)	Direction of Shear Loading	Ultimate Shear Load Ib (kN)
2-3/8	3 (76.2)	3 (76.2)	1,695 (7.5)	Any	2,080 (9.3)
(60)	12 (304.8)	12 (304.8)	2,425 (10.8)	Any	4,905 (21.8)
3-1/4 (83)	12 (304.8)	12 (304.8)	5,565 (24.8)	Any	7,944 (35.3)
	Embedment hom in. (mm) 2-3/8 (60) 3-1/4	Embedment hom in. (mm) Minimum Edge Distance in. (mm) 2-3/8 (60) 3 (76.2) 3 (76.2) 12 (304.8) 3-1/4 12	Embedment hnom in. (mm) Minimum Edge Distance in. (mm) Minimum Edge Distance in. (mm) 2-3/8 (60) 3 (76.2) 3 (76.2) 12 (304.8) 12 (304.8) 3-1/4 12	Embedment hnom in. (mm) Minimum Edge Distance in. (mm) Minimum Edg Distance in. (mm) Utimate Tension Load ib (kN) 2-3/8 (60) 3 (76.2) 3 (76.2) 1,695 (76.2) 2-3/8 (60) 12 (304.8) 12 (304.8) 2,425 (304.8) 3-1/4 12 12 5,565	Embedment hrom in. (mm) Minimum Edge Distance in. (mm) Minimum Edge Distance in. (mm) Unimate Lension Load ib (kN) Direction of Shear Loading 2-3/8 (60) 3 (76.2) 3 (76.2) 1.695 (76.2) Any 2-3/8 (60) 12 (304.8) 12 (304.8) Any 3-1/4 12 12 5,565

Tabulated load values are for anchors installed in minimum 8 inch wide, minimum Grade N, Type II, normal-weight concrete masonry units conforming to ASTM C 90. Mortar must be minimum Type N. Masonry compressive strength must be at the specified minimum at the time of installation.

2. Ultimate load capacities must be reduced by a minimum safety factor of 5.0 or greater to determine allowable working loads.

Allowable Load Capacities for Power-Stud+ SD4 and Power-Stud+ SD6 installed into the Face of Grout Filled Concrete Masonry^{1,2,3,4,5}

Nominal Anchor Diameter in.	Minimum Embedment h.om in. (mm)	Minimum Edge Distance in. (mm)	Minimum End Distance in. (mm)	Allowable Tension Load Ib (kN)	Direction of Shear Loading	Allowable Shear Load Ib (kN)
1/2	2-3/8	3 (76.2)	3 (76.2)	340 (1.5)	Any	415 (1.8)
1/2	(60)	12 (304.8)	12 (304.8)	485 (2.2)	Any	980 (4.4)
5/8	3-1/4 (83)	12 (304.8)	12 (304.8)	1,115 (5.0)	Any	1,590 (7.1)

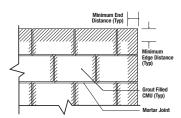
Tabulated load values are for anchors installed in minimum 8 inch wide, minimum Grade N, Type II, normal-weight concrete masonry units conforming to ASTM C 90. Mortar must be 1 minimum Type N. Masonry compressive strength must be at the specified minimum at the time of installation.

2. Allowable load capacities listed are calculated using an applied safety factor of 5.0. Consideration of safety factors of 10 or higher may be necessary depending upon the application such as life safety.

3. The tabulated values are applicable for anchors installed in grouted masonry wall faces at a critical spacing distance, ser, between anchors of 16 times the anchor diameter. The spacing distance between two anchors may be reduced to a minimum distance, smn, of 8 times the anchor diameter provided the allowable tension loads are multiplied a reduction factor of 0.80 and allowable shear loads are multiplied by a reduction factor of 0.90. Linear interpolation for calculation of allowable loads may be used for intermediate anchor spacing distances.

4. Anchors may be installed in the grouted cells and in cell webs and bed joints not closer than 1-3/8" from head joints. The minimum edge and end distances must also be maintained.

5. Allowable tension values for anchors installed into bed joints of grouted masonry wall faces with a minimum of 12" edge and end distance may be increased by 20 percent for the 1/2-inch diameter and 10 percent for the 5/8-inch diameter.



Wall Face **Permissible Anchor Locations** (Un-hatched Area)

STRENGTH DESIGN (SD)

Strength Design Installation Table for Power-Stud+ SD4 and Power-Stud+ SD6^{1,4}



						Nomina	al Anchor D	iameter					
Anchor Property/Setting Information	Notation	Units	1/4	3	/8	1	/2	5/	5/8		/4		
Anchor outside diameter	da	in. (mm)	0.250 (6.4)		375 .5)		500 2.7)	0.6 (15			750 9.1)		
Thread Size (UNC)	-	in.	1/4-20	3/8	3/8-16		-13	5/8-11		3/4	-10		
Minimum diameter of hole clearance in fixture	Сh	in. (mm)	5/16 (7.9)		(16 1.1)		9/16 (14.3)				/16 7.5)		/16).6)
Nominal drill bit diameter	dbit	in.	1/4 ANSI		3/8 1/2 5/8 ANSI ANSI ANSI				ANSI		/4 NSI		
Minimum nominal embedment depth ²	h _{nom}	in. (mm)	1-3/4 (44)		7/8 8)		1/2 64)	3- ⁻ (8	1/4 (3)		1/2 14)		
Effective embedment	h _{ef}	in. (mm)	1.50 (38)		50 88)	2. (5	00 51)	2. (7	75 0)		3/4 95)		
Minimum hole depth	h₀	in. (mm)	1-7/8 (48)		2 51)	2-5/8 (67)		3-1/2 (89)		4-3/4 (121)			
Minimum member thickness	h _{min}	in. (mm)	3-1/4 (83)	3-1/4 (83)	4 (102)		4 02)	5 (127)				6 52)	
Minimum overall anchor length ³	lanch	in. (mm)	2-1/4 (57)		3/4 '0)		3-3/4 4-1/2 (95) (114)		4-1/2		1/2 40)		
Minimum edge distance	Cmin	in. (mm)	1-3/4 (44)	3 (76)	3-1/2 (89)	6 (152)	3 (76)	4-1/2 (114)	8-1/2 (216)	5 (127)	9 (229)		
Minimum spacing distance	Smin	in. (mm)	2 (51)	5-1/2 (140)	3 (76)	3 (76)	6 (152)	8-1/2 (216)	5 (127)	9 (229)	5 (127)		
Critical edge distance	Cac	in. (mm)	5 (127)		5 27)	7- (1	1/2 91)	9-1 (24	1/2 41)		9 29)		
Installation torque	Tinst	ftlbf. (N-m)	6 (8)		25 34)		0 64)	6 (8	-		10 49)		
Torque wrench/socket size	-	in.	7/16	9/	16	3	/4	15	/16	1-	1/8		
Nut height	-	in.	7/32	21	/64	7/	16	35,	/64	41	/64		

For SI: 1 inch = 25.4 mm; 1 ft-lbf = 1.356 N-m.

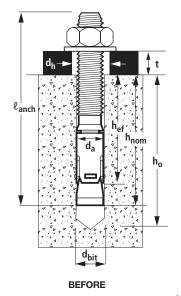
1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable.

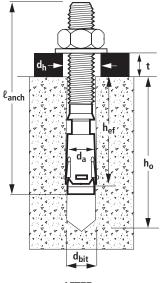
2. The embedment depth, hnom, is measured from the outside surface of the concrete member to the embedded end of the anchor prior to tightening.

3. The listed minimum overall anchor length is based on anchor sizes commercially available at the time of publication compared with the requirements to achieve the minimum nominal embedment depth and possible fixture attachment.

4. The anchors may be installed in the topside of concrete-filled steel deck floor and roof assemblies in accordance with the following: the 1/4-inch diameter anchors must be installed in uncracked normal-weight or sand-lightweight concrete; 3/8-inch to 3/4-inch diameter anchors must be installed in cracked and uncracked normal-weight or sand-lightweight concrete over steel deck having a minimum specified compressive strength, f'c, of 3,000 psi (20.7 MPa) provided the concrete thickness above the upper flute meets the minimum thickness specified in this table.

Power-Stud+ SD4 and Power-Stud+ SD6 Anchor Detail





AFTER

Application of Installation Torque

7

ANCHORS

CHANICAL ,

Tension Design Information for Power-Stud+ SD4 and Power-Stud+ SD6 Anchors in Concrete (For use with load combinations taken from ACI 318-14, Section 5.3 or ACI 318-11, Section 9.2)^{1,8}



Docian Ch	aractoristic	Notation	Units		Nominal Anchor Diamete							
Design Ch	Design Characteristic		Units	1/4	3/8	1/2	5/8	3/4				
Anchor category		1,2 or 3	-	1	1	1	1	1				
Nominal embedment dep	oth	h _{nom}	in.	1-3/4	1-7/8	2-3/8	3-1/4	4-1/2				
	STE	EL STRENGTH	IN TENSION	I (ACI 318-14 17.4	1 or ACI 318-11 D	.5.1)						
Minimum specified yield	strength (neck	fy	ksi (N/mm²)	60 (414)	60 (414)	60 (414)	60 (414)	60 (414)				
Minimum specified ultimation	ate tensile strength (neck)	f _{uta}	ksi (N/mm²)	90 (621)	90 (621)	90 (621)	90 (621)	90 (621)				
Effective tensile stress ar	ea (neck)	A _{se,N}	in² (mm²)	0.0249 (16.1)	0.0530 (34.2)	0.1020 (65.8)	0.1630 (105.2)	0.2380 (151)				
Steel strength in tension		N _{sa}	lb (kN)	2,240 (10.0)	4,780 (21.3)	9,160 (40.8)	14,635 (65.1)	21,380 (95.1)				
Reduction factor for steel	strength ^{2,3}	ϕ	-			0.75						
	CONCRETE I	BREAKOUT ST	RENGTH IN 1	TENSION (ACI 318-	14 17.4.2 or ACI 3	18-11 D.5.2) [®]						
Effective embedment		h _{ef}	in. (mm)	1.50 (38)	1.50 (38)	2.00 (51)	2.75 (70)	3.75 (95)				
Effectiveness factor for u	ncracked concrete	Kuncr	-	24	24	24	24	24				
Effectiveness factor for c	racked concrete	k _{cr}	-	Not Applicable	17	21	21	21				
Modification factor for cracked and uncracked of	concrete	Ψc,N	-	1.0 See Note 5	1.0 See Note 5	1.0 See Note 5	1.0 See Note 5	1.0 See Note 5				
Critical edge distance (ur	ncracked concrete only)	Cac	in. (mm)	5 (127)	5 (127)	7-1/2 (191)	9 (229)					
Reduction factor for cond	crete breakout strength⁴	ϕ	-		0.65 (Condition B)							
	PULL	DUT STRENGT	h in tensio	N (ACI 318-14 17.4	4.3 or ACI 318-11	D.5.3)°						
Characteristic pullout stre uncracked concrete (2,50	ength, D0 psi)⁵	N _{p,uncr}	lb (kN)	1,510 (6.7)	See Note 7	See Note 7	See Note 7	8,520 (37.8)				
Characteristic pullout stre cracked concrete (2,500	ength, psi)⁵	N _{p,cr}	lb (kN)	Not Applicable	See Note 7	See Note 7	See Note 7	See Note 7				
Reduction factor for pullo	out strength ³	ϕ	-	0.65 (Condition B)								
	PULLOUT STRENGTH I	N TENSION FO	R SEISMIC	APPLICATIONS (ACI	318-14 17.2.3.3	or ACI 318-11 D.3.3	3.3) ⁸					
Characteristic pullout stre	ngth, seismic (2,500 psi) ^{6,9}	N _{p,eq}	lb (kN)	Not Applicable 1,645 (7.3)		See Note 7	See Note 7	See Note 7				
Reduction factor for pullo	out strength⁴	ϕ	-			0.65 (Condition B)						
Mean axial stiffnes	Uncracked concrete	β	lbf/in (kN/mm)	171,400 (30,060)	490,000 (86,000)	459,000 (80,500)	234,000 (41,000)	395,000 (69,300)				
values for service load range	Cracked concrete	β	lbf/in (kN/mm)	Not Applicable	228,000 (40,000)	392,000 (68,800)	193,000 (33,800)	76,600 (13,400)				

1. The data in this table is intended to be used with the design provisions of ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.

2. The tabulated value of ϕ for steel strength applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ for steel strength must be determined in accordance with ACI 318-11 D.4.4.

3. The anchors are ductile steel elements as defined in ACI 318-14 2.3 or ACI 318-11 D.1, as applicable.

4. The tabulated value of ϕ for concrete breakout strength and pullout strength applies when both the load combinations of Section 1605.2 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-14 17.3.3 or ACI 318-11 D.4.3, for Condition B are satisfied. If the load combinations of Section 1605.2 of the IBC ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-14 17.3.3 or ACI 318-11 D.4.3, for Condition A are satisfied, the appropriate value of ϕ for concrete breakout strength and pullout strength must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ for concrete breakout strength and pullout strength must be determined in accordance with ACI 318-11 D.4.4.

5. For all design cases $\psi_{c,N} = 1.0$. The appropriate effectiveness factor for cracked concrete (kor) or uncracked concrete (kurror) must be used.

6. For all design cases $\psi_{c,P} = 1.0$. For concrete compressive strength greater than 2,500 psi, N_Pn = (pullout strength value from table)*(specified concrete compressive strength/2,500)^{6.5}.

7. Pullout strength does not control design of indicated anchors. Do not calculate pullout strength for indicated anchor size and embedment.

8. Anchors are permitted to be used in lightweight concrete provided the modification factor λ_{a} equal to 0.8 λ is applied to all values of $\sqrt{t^{r}c}$ affecting N_n and V_n. λ shall be determined in accordance with the corresponding version of ACI 318.

9. Tabulated values for characteristic pullout strength in tension are for seismic applications and are based on test results per ACI 355.2, Section 9.5.

10. Actual stiffness of the mean value varies depending on concrete strength, loading and geometry of application.

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Shear Design Information for Power-Stud+ SD4 and Power-Stud+ SD6 Anchors in Concrete (For use with load combinations taken from ACI 318-14, Section 5.3 or ACI 318-11, Section 9.2)¹⁷



	Natalian		Nominal Anchor Diameter							
Design Characteristic	Notation	Units	1/4	3/8	1/2	5/8	3/4			
Anchor category	1, 2 or 3	-	1	1	1	5/8 1 3-1/4 60 (414) 90 (621) 0.226 (145.8) 7,455 (33.6) 2.75 (69.9) 0.625 (15.9) B) 2.0 2.75 (69.9) B)	1			
Nominal embedment depth	hnom	in.	1-3/4	1-7/8	2-3/8	3-1/4	4-1/2			
	STEEL STRENG	GTH IN SHEAR (AC	l 318-14 17.5.1 or	ACI 318-11 D.6.1)4					
Minimum specified yield strength (threads)	fy	ksi (N/mm²)	60 (414)	60 (414)	60 (414)		60 (414)			
Minimum specified ultimate strength (threads)	f _{uta}	ksi (N/mm²)	90 (621)	90 (621)	90 (621)		90 (621)			
Effective tensile stress area (threads)	A _{se, v} [Ase] ⁸	in² (mm²)	0.0318 (20.5)	0.078 (50.3)	0.142 (91.6)		0.334 (212)			
Steel strength in shear ⁶	Vsa	lb (kN)	1,115 (5.0)	1,470 (6.6)	3,170 (14.3)		11,955 (53.2)			
Reduction factor for steel strength ^{2,3}	ϕ	-			0.65					
CONCF	ETE BREAKOUT	STRENGTH IN SH	EAR (ACI 318-14 1	7.5.2 or ACI 318-	11 D.6.2)					
Load bearing length of anchor $(h_{ef} \text{ or } 8d_{a}, \text{ whichever is less})$	le	in. (mm)	1.50 (38.1)	1.50 (38.1)	2.00 (50.8)		3.75 (95)			
Nominal anchor diameter	da	in. (mm)	0.250 (6.4)	0.375 (9.5)	0.500 (12.7)		0.750 (19.1)			
Reduction factor for concrete breakout ⁴	ϕ	-		0.70 (Condition B)						
CONC	RETE PRYOUT S	TRENGTH IN SHEA	R (ACI 318-14 17.2	2.3.3 or ACI 318-	11 D.6.3)					
Coefficient for pryout strength (1.0 for $h_{ef} < 2.5$ in., 2.0 for $h_{ef} \ge 2.5$ in.)	k _{cp}	-	1.0	1.0	1.0	2.0	2.0			
Effective embedment	hef	in. (mm)	1.50 (38.1)	1.50 (38.1)	2.00 (50.8)		3.75 (95)			
Reduction factor for pryout strength ⁵	ϕ	-			0.70 (Condition B)	-				
STEEL STRENG	TH IN SHEAR FO	R SEISMIC APPLI	CATIONS (ACI 318-	14 17.2.3.3 or A0	CI 318-11 D.3.3.3)					
Steel strength in shear, seismic [®]	V _{sa,eq}	lb (kN)	Not Applicable	1,305 (5.9)	2,765 (12.3)		7,745 (34.5)			
Reduction factor for steel strength in shear for seismic ²	φ	-			0.65					

1. The data in this table is intended to be used with the design provisions of ACI 318-14 Chapter or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.

2. The tabulated value of ϕ for steel strength applies when the load combinations of Section 1605.2 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11Section 9.2, as applicable, are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ for steel strength must be determined in accordance with ACI 318-11 D.4.4.

3. The anchors are ductile steel elements as defined in ACI 318-14 2.3 or ACI 318-11 D.1, as applicable.

4. The tabulated value of φ for concrete breakout strength applies when both the load combinations of Section 1605.2 of the IBC, ACI 318-14 Section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-14 17.3.3 or ACI 318-11 D.4.3, for Condition B are satisfied. If the load combinations of Section 1605.2 of the IBC, ACI 318-11 Section 9.2, as section 5.3 or ACI 318-11 Section 9.2, as applicable, are used and the requirements of ACI 318-11 D.4.3, for Condition B are satisfied. If the load combinations of Section 1605.2 of the IBC, ACI 318-14 IS. Section 9.2, as applicable, are used and the requirements of ACI 318-14 14.3.3 or ACI 318-11 D.4.3, for Condition A are satisfied, the appropriate value of φ for concrete breakout strength must be determined in accordance with ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of φ for concrete breakout strength must be determined in accordance with ACI 318-11 D.4.3.

5. The tabulated value of for pryout strength applies if the load combinations of Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 Section 9.2 are used. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ for pryout strength must be determined in accordance with ACI 318-11 D.4.4, Condition B.

6. Tabulated values for steel strength in shear must be used for design.

7. Anchors are permitted to be used in lightweight concrete provided the modification factor λ_{a} equal to 0.8 λ is applied to all values of $\sqrt{f'c}$ affecting Nn and Vn. λ shall be determined in accordance with the corresponding version of ACI 318.

8. Tabulated values for steel strength in shear are for seismic applications are based on test results per ACI 355.2, Section 9.6.

STRENGTH DESIGN PERFORMANCE DATA

Factored design strength ϕ Nn and ϕ Vn Calculated in accordance with ACI 318-14 Chapter 17 Compliant with the International Building Code

• F. '.'/ /

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Tension and Shear Design Strengths Installed in Cracked Concrete¹⁻⁶

			Minimum Concrete Compressive Strength									
Nominal			f'c = 2,500 psi		f'c = 3,000 psi		000 psi	f'C = 6,	000 psi	f'c = 8,000 psi		
Anchor Diameter (in.)	Embed. hnom (in.)	$\phi_{N_{sa}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi_{V_{sa}}, \phi_{V_{cb}}$ or $\phi_{V_{cp}}$ Shear (lbs.)	φ _{Nsa} , φ _{Ncb} or φ _{Ncp} Tension (lbs.)	ϕ V _{sa} , ϕ V _{cb} or ϕ V _{cp} Shear (lbs.)	$\phi_{N_{sa}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi_{V_{sa}}, \phi_{V_{cb}}$ or $\phi_{V_{cp}}$ Shear (lbs.)	$\phi_{N_{sa}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi V_{sa}, \phi V_{cb}$ or ϕV_{cp} Shear (lbs.)	$\phi_{N_{sa}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	φVsa, φVcb or φVcp Shear (lbs.)	
1/4	-	-	-	-	-	-	-	-	-	-	-	
3/8	1-7/8	1,015	955	1,110	955	1,285	955	1,570	955	1,815	955	
1/2	2-1/2	1,930	2,060	2,115	2,060	2,440	2,060	2,990	2,060	3,455	2,060	
5/8	3-1/4	3,110	4,520	3,410	4,845	3,935	4,845	4,820	4,845	5,570	4,845	
3/4	4-1/2	4,955	5,270	5,430	5,770	6,270	6,665	7,680	7,770	8,865	7,770	
Anchor Pu	llout/Priout Stror	ath Controls 🗖	Concroto Broa	kout Strongth Co	ntrole 🗖 Stool	Strongth Control	0					

🔲 - Anchor Pullout/Pryout Strength Controls 🔲 - Concrete Breakout Strength Controls 📕 - Steel Strength Controls

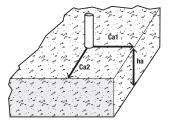
Tension and Shear Design Strengths Installed in Uncracked Concrete¹⁻⁶

		Minimum Concrete Compressive Strength										
Nominal			f'c = 2,500 psi		f'c = 3,000 psi		000 psi	f'c = 6,	000 psi	f'c = 8,000 psi		
Anchor Diameter (in.)	Embed. hnorm (in.)	$\phi_{N_{say}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi_{V_{sa}}, \phi_{V_{cb}}$ or $\phi_{V_{cp}}$ Shear (lbs.)	$\phi_{N_{say}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi_{V_{sa}}, \phi_{V_{cb}}$ or $\phi_{V_{cp}}$ Shear (lbs.)	$\phi_{N_{sa}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi_{V_{sa}}, \phi_{V_{cb}}$ or $\phi_{V_{cp}}$ Shear (lbs.)	$\phi_{N_{sa}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	$\phi_{V_{sa}}, \phi_{V_{cb}}$ or $\phi_{V_{cp}}$ Shear (lbs.)	$\phi_{N_{say}}, \phi_{N_{cb}}$ or $\phi_{N_{cp}}$ Tension (lbs.)	φV _{sa} , φV _{cb} or φV _{cp} Shear (lbs.)	
1/4	1-3/4	980	725	1,075	725	1,240	725	1,520	725	1,680	725	
3/8	1-7/8	1,435	955	1,570	955	1,815	955	2,220	955	2,565	955	
1/2	2-1/2	2,205	2,060	2,415	2,060	2,790	2,060	3,420	2,060	3,945	2,060	
5/8	3-1/4	3,555	4,845	3,895	4,845	4,500	4,845	5,510	4,845	6,365	4,845	
3/4	4-1/2	5,540	7,375	6,065	7,770	7,005	7,770	8,580	7,770	9,905	7,770	
Anobor Du		acth Controla	,	,		,	1					

🗖 - Anchor Pullout/Pryout Strength Controls 🔲 - Concrete Breakout Strength Controls 📕 - Steel Strength Controls

1- Tabular values are provided for illustration and are applicable for single anchors installed in normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions: - c_{at} is greater than or equal to the critical edge distance, c_{ac} (table values based on $c_{at} = c_{ac}$).

- c_{a2} is greater than or equal to 1.5 times c_{a1}.
 Calculations were performed according to ACI 318-14 Chapter 17. The load level corresponding to the controlling failure mode is listed. (e.g. For tension: steel, concrete breakout and pullout; For shear: steel,
- 22 Controlling failure mode is listed. (e.g. For tension: steel, concrete breakout and pullout; For shear: steel, concrete breakout and pryout). Furthermore, the capacities for concrete breakout strength in tension and pryout strength in shear are calculated using the effective embedment values, her, for the selected anchors as noted in the design information tables. Please also reference the installation specifications for more information.
- 3- Strength reduction factors (ø) were based on ACI 318-14 Section 5.3 for load combinations. Condition B is assumed.
- 4- Tabular values are permitted for static loads only, seismic loading is not considered with these tables.
- 5- For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-14 Chapter 17.
- 6- Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths please see ACI 318-14 Chapter 17. For other design conditions including seismic considerations please see ACI 318-14 Chapter 17.







ORDERING INFORMATION

All anchors are packaged with nuts and washers. A manual hand pump is available (Cat. No. 08280).

Hollow drill bits must be used with a dust extraction vacuum (Cat. No. DW012).

Power-Stud+ SD4 (Type 304 Stainless Steel Body) and Power-Stud+ SD6 (Type 316 Stainless Steel Body)

ype state s	Anchor Size 1/4" x 1-3/4" 1/4" x 2-1/4" 1/4" x 3-1/4" 3/8" x 2-1/4" 3/8" x 2-3/4" 3/8" x 3" 3/8" x 3-1/2"	Thread Length 3/4" 1-1/4" 2-1/4" 7/8" 1-3/8" 1-5/8"	Box Qty. 100 100 100 50 50	Carton Qty. 600 600 600 300	Full Head SDS-Plus DW5517 DW5517 DW5517	SDS-Plus DW5416 DW5417 DW5417	SDS-Max - -	Hollow Bit SDS-Plus -	Hollow Bit SDS-Max
2SD6 4SD6 0SD6 2SD6 3SD6 4SD6 5SD6	1/4" x 2-1/4" 1/4" x 3-1/4" 3/8" x 2-1/4" 3/8" x 2-3/4" 3/8" x 3" 3/8" x 3-1/2"	1-1/4" 2-1/4" 7/8" 1-3/8" 1-5/8"	100 100 50	600 600	DW5517 DW5517	DW5417	-	-	-
ASD6 OSD6 2SD6 3SD6 4SD6 5SD6	1/4" x 3-1/4" 3/8" x 2-1/4" 3/8" x 2-3/4" 3/8" x 3" 3/8" x 3"	2-1/4" 7/8" 1-3/8" 1-5/8"	100 50	600	DW5517			-	-
0SD6 2SD6 3SD6 4SD6 5SD6	3/8" x 2-1/4" 3/8" x 2-3/4" 3/8" x 3" 3/8" x 3"	7/8" 1-3/8" 1-5/8"	50			DW5417			
2SD6 3SD6 4SD6 5SD6	3/8" x 2-3/4" 3/8" x 3" 3/8" x 3-1/2"	1-3/8" 1-5/8"		300		DWOTH	-	-	-
3SD6 4SD6 5SD6	3/8" x 3" 3/8" x 3-1/2"	1-5/8"	50		DW5527	DW5427	-	-	-
4SD6 5SD6	3/8" x 3-1/2"			300	DW5527	DW5427	-	-	-
5SD6		1	50	300	DW5527	DW5427	-		
		2-1/8"	50	300	DW5527	DW5427	-	-	-
6SD6	3/8" x 3-3/4"	2-3/8"	50	300	DW5527	DW5427	-	-	-
	3/8" x 5"	3-5/8"	50	300	DW55300	DW5429	-	-	-
7SD6	3/8" x 7"	5-5/8"	50	200	DW55300	DW5429	-	-	-
20SD6	1/2" x 2-3/4"	1"	50	200	DW5537	DW5437	DW5803	DWA54012	-
2SD6	1/2" x 3-3/4"	2"	50	200	DW5537	DW5437	DW5803	DWA54012	-
3SD6	1/2" x 4-1/2"	2-3/4"	50	200	DW5539	DW5438	DW5803	DWA54012	-
4SD6	1/2" x 5-1/2"	3-3/4"	50	100	DW5539	DW5438	DW5803	DWA54012	-
6SD6	1/2" x 7"	5-1/4"	25	100	DW5539	DW5438	DW5803	DWA54012	-
BOSD6	5/8" x 3-1/2"	1-1/2"	25	100	-	DW5446	DW5806	DWA54058	DWA58058
32SD6	5/8" x 4-1/2"	2-1/2"	25	100	-	DW5446	DW5806	DWA54058	DWA58058
3SD6	5/8" x 5"	3"	25	100	-	DW5446	DW5806	DWA54058	DWA58058
34SD6	5/8" x 6"	4"	25	75	-	DW5446	DW5806	DWA54058	DWA58058
6SD6	5/8" x 7"	5"	25	75	-	DW5447	DW5806	DWA54058	DWA58058
8SD6	5/8" x 8-1/2"	6-1/2"	25	50	-	DW5447	DW5809	DWA54058	DWA58058
OSD6	3/4" X 4-1/4"	1-7/8"	20	60	-	DW5453	DW5810	DWA54034	DWA58034
1SD6	3/4" X 4-3/4"	2-3/8"	20	60	-	DW5453	DW5810	DWA54034	DWA58034
2SD6	3/4" X 5-1/2"	3-1/8"	20	60	-	DW5453	DW5810	DWA54034	DWA58034
4SD6	3/4" X 6-1/4"	3-7/8"	20	60	-	DW5455	DW5810	DWA54034	DWA58034
6SD6	3/4" X 7"	4-5/8"	20	60	-	DW5455	DW5810	DWA54034	DWA58034
8SD6	3/4" X 8-1/2"	6-1/8"	10	40	-	DW5455	DW5812	DWA54034	DWA58034
I9SD6	3/4" x 10"	7-5/8"	10	40	-	DW5455	DW5812	DWA54034	DWA58034
all for detand	ails. ote sizes which are les	s than the m	iinimum st	andard ancl			n and domestic	components) and	are
22222222222222222222222222222222222222	ISD6 ISD6 ISD6 ISD6 ISD6 ISD6 ISD6 ISD6	SSD6 3/8" x 5" 7SD6 3/8" x 7" DSD6 1/2" x 2-3/4" SSD6 1/2" x 3-3/4" SSD6 1/2" x 4-1/2" SSD6 1/2" x 5-1/2" SSD6 1/2" x 5-1/2" SSD6 1/2" x 5-1/2" SSD6 1/2" x 5-1/2" SSD6 5/8" x 4-1/2" SSD6 5/8" x 4-1/2" SSD6 5/8" x 6" SSD6 5/8" x 6" SSD6 5/8" x 8-1/2" SSD6 5/8" x 8-1/2" SSD6 3/4" x 4-3/4" SD6 3/4" x 5-1/2" SSD6 3/4" x 4-3/4" SD6 3/4" x 7" SSD6 3/4" x 8-1/2" SD6 3/4" x 8-1/2" SD6 3/4" x 10" nd Power-Stud+ SD6 anchors c Il for details. bers denote sizes which are less	SSD6 3/8" x 5" 3-5/8" SSD6 3/8" x 7" 5-5/8" SSD6 1/2" x 2-3/4" 1" SSD6 1/2" x 3-3/4" 2" SSD6 1/2" x 4-1/2" 2-3/4" SSD6 1/2" x 5-1/2" 3-3/4" SSD6 1/2" x 5-1/2" 3-3/4" SSD6 1/2" x 7" 5-1/4" SSD6 1/2" x 5-1/2" 3-3/4" SSD6 1/2" x 5-1/2" 3-3/4" SSD6 5/8" x 4-1/2" 2-1/2" SSD6 5/8" x 4-1/2" 2-1/2" SSD6 5/8" x 6" 4" SSD6 5/8" x 7" 5" SSD6 5/8" x 8-1/2" 6-1/2" SSD6 3/4" x 4-1/4" 1-7/8" SD6 3/4" x 4-1/4"	SSD6 $3/8" \times 5"$ $3-5/8"$ 50 SSD6 $3/8" \times 7"$ $5-5/8"$ 50 DSD6 $1/2" \times 2-3/4"$ $1"$ 50 DSD6 $1/2" \times 2-3/4"$ $1"$ 50 DSD6 $1/2" \times 3-3/4"$ $2"$ 50 DSD6 $1/2" \times 3-3/4"$ $2"$ 50 DSD6 $1/2" \times 3-3/4"$ $2"$ 50 DSD6 $1/2" \times 5-1/2"$ $2-3/4"$ 50 DSD6 $1/2" \times 5-1/2"$ $3-3/4"$ 50 DSD6 $5/8" \times 5-1/2"$ $3-3/4"$ 50 DSD6 $5/8" \times 3-1/2"$ $1-1/2"$ 25 DSD6 $5/8" \times 3-1/2"$ $2-1/2"$ 25 DSD6 $5/8" \times 5"$ $3"$ 25 DSD6 $5/8" \times 8-1/2"$ $6-1/2"$ 25 DSD6 $3/4" \times 4-3/4"$ $1-7/8"$ 20 DSD6 $3/4" \times 4-3/4"$ $2-3/8"$ 20 SD6 $3/4" \times 5-1/2"$ $3-1/8"$ 20 <t< td=""><td>SSD6 3/8" x 5" 3-5/8" 50 300 7SD6 3/8" x 7" 5-5/8" 50 200 0SD6 1/2" x 2-3/4" 1" 50 200 2SD6 1/2" x 2-3/4" 1" 50 200 2SD6 1/2" x 3-3/4" 2" 50 200 2SD6 1/2" x 4-1/2" 2-3/4" 50 200 SD6 1/2" x 5-1/2" 3-3/4" 50 100 SD6 1/2" x 5-1/2" 3-3/4" 50 100 SD6 5/8" x 5-1/2" 3-3/4" 50 100 SD6 5/8" x 3-1/2" 1-1/2" 25 100 SD6 5/8" x 4-1/2" 2-1/2" 25 100 SD6 5/8" x 5" 3" 25 100 SD6 5/8" x 6" 4" 25 75 SD6 5/8" x 8-1/2" 6-1/2" 25 50 SD6 3/4" X 4-3/4" 2-3/8" 20 60 SD6</td><td>SSD6 3/8" x 5" 3-5/8" 50 300 DW55300 7SD6 3/8" x 7" 5-5/8" 50 200 DW55300 0SD6 1/2" x 2-3/4" 1" 50 200 DW5537 2SD6 1/2" x 3-3/4" 2" 50 200 DW5537 2SD6 1/2" x 3-3/4" 2" 50 200 DW5537 3SD6 1/2" x 4-1/2" 2-3/4" 50 200 DW5539 SD6 1/2" x 5-1/2" 3-3/4" 50 100 DW5539 SD6 1/2" x 5-1/2" 3-3/4" 50 100 DW5539 SD6 5/8" x 3-1/2" 1-1/2" 25 100 - SD6 5/8" x 4-1/2" 2-1/2" 25 100 - SD6 5/8" x 6" 4" 25 75 - SD6 5/8" x 8-1/2" 6'1/2" 25 50 - SD6 5/8" x 8-1/2" 6-1/2" 25 50 -</td><td>SSD6 3/8" x 5" 3-5/8" 50 300 DW55300 DW5429 YSD6 3/8" x 7" 5-5/8" 50 200 DW55300 DW5429 DSD6 1/2" x 2-3/4" 1" 50 200 DW5537 DW5437 QSD6 1/2" x 3-3/4" 2" 50 200 DW5537 DW5437 QSD6 1/2" x 4-1/2" 2-3/4" 50 200 DW5539 DW5438 QSD6 1/2" x 5-1/2" 3-3/4" 50 100 DW5539 DW5438 QSD6 1/2" x 7" 5-1/4" 25 100 DW5539 DW5438 QSD6 5/8" x 3-1/2" 1-1/2" 25 100 - DW5446 QSD6 5/8" x 4-1/2" 2-1/2" 25 100 - DW5446 QSD6 5/8" x 6" 4" 25 75 - DW5446 QSD6 5/8" x 8-1/2" 6-1/2" 25 50 - DW5447 QSD6 5/8" x 8-1/2"<</td><td>SSD6 3/8" x 5" 3-5/8" 50 300 DW55300 DW5429 - SSD6 3/8" x 7" 5-5/8" 50 200 DW55300 DW5429 - SSD6 1/2" x 2-3/4" 1" 50 200 DW5537 DW5437 DW5803 SSD6 1/2" x 3-3/4" 2" 50 200 DW5539 DW5437 DW5803 SSD6 1/2" x 4-1/2" 2-3/4" 50 200 DW5539 DW5438 DW5803 SSD6 1/2" x 5-1/2" 3-3/4" 50 100 DW5539 DW5438 DW5803 SSD6 1/2" x 7" 5-1/4" 25 100 - DW5446 DW5806 SSD6 5/8" x 4-1/2" 2-1/2" 25 100 - DW5446 DW5806 SSD6 5/8" x 4" 12" 25 75 - DW5446 DW5806 SSD6 5/8" x 8" 4" 25 75 - DW5447 DW5806</td><td>SDG 3/8" x 5" 3-5/8" 50 300 DW55300 DW5429 - - 'SDG 3/8" x 7" 5-5/8" 50 200 DW55300 DW5429 - - \SD6 1/2" x 2-3/4" 1" 50 200 DW5537 DW5437 DW5803 DWA54012 \SD6 1/2" x 3-3/4" 2" 50 200 DW5539 DW5438 DW5803 DWA54012 \SD6 1/2" x 4-1/2" 2-3/4" 50 200 DW5539 DW5438 DW5803 DWA54012 \SD6 1/2" x 7" 5-1/4" 25 100 DW5539 DW5438 DW5803 DWA54012 \SD6 1/2" x 7" 5-1/4" 25 100 - DW5438 DW5803 DWA54012 \SD6 5/8" x 3-1/2" 1-1/2" 25 100 - DW5446 DW5806 DWA54058 \SD6 5/8" x 4-1/2" 2-1/2" 25 75 - DW5446 DW5806 DWA54058</td></t<>	SSD6 3/8" x 5" 3-5/8" 50 300 7SD6 3/8" x 7" 5-5/8" 50 200 0SD6 1/2" x 2-3/4" 1" 50 200 2SD6 1/2" x 2-3/4" 1" 50 200 2SD6 1/2" x 3-3/4" 2" 50 200 2SD6 1/2" x 4-1/2" 2-3/4" 50 200 SD6 1/2" x 5-1/2" 3-3/4" 50 100 SD6 1/2" x 5-1/2" 3-3/4" 50 100 SD6 5/8" x 5-1/2" 3-3/4" 50 100 SD6 5/8" x 3-1/2" 1-1/2" 25 100 SD6 5/8" x 4-1/2" 2-1/2" 25 100 SD6 5/8" x 5" 3" 25 100 SD6 5/8" x 6" 4" 25 75 SD6 5/8" x 8-1/2" 6-1/2" 25 50 SD6 3/4" X 4-3/4" 2-3/8" 20 60 SD6	SSD6 3/8" x 5" 3-5/8" 50 300 DW55300 7SD6 3/8" x 7" 5-5/8" 50 200 DW55300 0SD6 1/2" x 2-3/4" 1" 50 200 DW5537 2SD6 1/2" x 3-3/4" 2" 50 200 DW5537 2SD6 1/2" x 3-3/4" 2" 50 200 DW5537 3SD6 1/2" x 4-1/2" 2-3/4" 50 200 DW5539 SD6 1/2" x 5-1/2" 3-3/4" 50 100 DW5539 SD6 1/2" x 5-1/2" 3-3/4" 50 100 DW5539 SD6 5/8" x 3-1/2" 1-1/2" 25 100 - 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